

Midpeninsula Regional Open Space District

R-20-19 Meeting 20-03 February 12, 2020

# AGENDA ITEM

# AGENDA ITEM 6

Contract Amendment for Engineering Services with Wiss, Janney, Elstner Associates, Inc., for the Mount Umunhum Radar Tower Repair Project

# GENERAL MANAGER'S RECOMMENDATIONS

Authorize a contract amendment in the amount of \$89,975 with Wiss, Janney, Elstner Associates, Inc., of Emeryville, California to provide construction documents, construction administration, and as-built drawings for the Mount Umunhum Radar Tower Repair Project, bringing the contract to a not-to-exceed grand total amount of \$213,830.

# SUMMARY

On November 20, 2019, the Board of Directors (Board) directed the General Manager to return with a recommended award of contract to develop construction documents for the Board-selected Long-Term Repair option to complete the Mount Umunhum Radar Tower Repair Project (Repair Project). Based on the results of a competitive professional services selection process completed at the start of this project, Wiss, Janney, Elstner Associates, Inc., (WJE) was selected as the best qualified firm to complete the Phase I assessment and Basis of Design work at a fair price. Given WJE's technical qualifications and relevant experience of the firm, the efficient and high quality Phase I work product that yielded a substantial contract savings (\$33,425 or nearly 30% of the base contract), and institutional knowledge of the project, the General Manager recommends amending WJE's contract in the amount of \$89,975, for a not-to-exceed total contract amount of \$213,830. This amount reflects Midpeninsula Regional Open Space District (District) staff diligence in negotiating a fair and reasonable price with WJE for Phase II work. There are sufficient funds in the Fiscal Year (FY) 2019-20 project budget to cover the recommended action and expenditures. Funding to construct the repairs will be proposed as part of the upcoming annual Budget and Action Plan process.

# BACKGROUND

In 1986, the District acquired the former Almaden Air Force Station (Almaden AFS) and all of its remaining facilities on Mount Umunhum and Mount Thayer with the intent to restore the area and provide public access (R-86-20). In December 2009, the United States Congress appropriated \$3.2 million for the identification, evaluation, and remediation of hazardous materials and site cleanup at the former Almaden AFS site, which included abatement of the Mount Umunhum radar tower. In 2011, the United States Army Corp of Engineers completed the removal of at-risk and peeling lead-containing paint from the exterior of the radar tower, as well as removal of unstable lead-containing paint and other hazardous materials from the interior. In 2014, following the federally-funded remediation project, the District completed site

demolition and landform restoration at the former Almaden AFS site (R-12-90). Through this work, the District restored much of the natural mountaintop, leaving only the radar tower.

On January 28, 2015, the Board authorized short-term interim structural repairs to the radar tower prior to making a final decision on the future disposition of the structure (R-15-09). On May 10, 2016, the County Board of Supervisors listed the radar tower on the County Heritage Resource Inventory. Given this action and implications on the future disposition of the radar tower, the Board approved the Retain and Seal option for the structure on June 8, 2016 (R-16-75). The Retain and Seal approach effectively calls for the sealing and stabilization of the structure, with no visitor access to the interior. See Attachment 2 for additional project history.

On September 12, 2018 (R-18-105), the Board approved the following goals and criteria to guide the development of additional repairs for the Mount Umunhum Radar Tower Repair Project:

- Ensure public and worker safety around the radar tower
- Avoid future contamination concerns
- Reduce (or eliminate) future need to enter building
- Protect workers if/when ingress is needed
- Avoid wildlife trappings and other resource impacts
- Reopen a pathway to the east summit
- Supports the retain and seal option previously approved by the Board of Directors

On January 23, 2019, the Board authorized the General Manager to enter into contract with WJE to complete a comprehensive radar tower assessment (structural, safety, and hazardous materials) and develop short-term and long-term repair options that meet the Board-approved goals and criteria (R-19-09).

On November 20, 2019, the Board selected the Long-Term Repairs option to stabilize and extend the life span of the radar tower, and directed the General Manager to return to the Board with a recommended contract to complete the construction documents and assist with construction administration to complete the selected Long-Term Repair option (R-19-155).

# DISCUSSION

A Request for Proposals (RFP) was issued in October 2018 to solicit qualifications and proposals for Phase I of the project: to assess the radar tower and develop short and long-term repair options. The RFP also stated that the Project would be divided into two phases, and that the consultant selected to complete Phase I may also be selected to complete Phase II: to develop construction documents and provide construction administration services.

Through the RFP process, WJE was deemed the most qualified firm given the team's prior experience with similar concrete military buildings of historic interest, and expertise in the field of structural engineering and concrete structure evaluation. WJE also offered competitive rates, and the lowest cost proposal to complete Phase I. For these reasons, the Board awarded the Phase I contract to WJE on January 23, 2019.

Throughout Phase I, WJE has demonstrated strong knowledge of appropriate concrete structure repair options. WJE also was highly responsive to District requirements and the Board-approved projects goals and criteria. Moreover, WJE completed the Phase I scope significantly under

budget, with a cost savings of \$33,425 from the base contract (30% savings – does not include untouched contingency) indicating their high level of efficiency and expertise that aids in streamlining the completion of work products for this type of structure. WJE continues to be highly qualified to perform Phase II. For all these reasons, staff proceeded to negotiate directly with WJE for the Phase II work, and arrived at a fair and reasonable fee. Together, the cumulative fee for both phases represents approximately 12% of the total estimated construction cost. This proportion of soft costs as compared to construction hard costs falls below industry standards (typically 15% to as high as 30%)

Therefore, the General Manager recommends amending WJE's existing contract by an amount of \$89,975 to complete the Phase II work: construction documents, construction administration, and as-built drawings. With the cost savings that remains in the contract from Phase I and the additional contract amendment amount requested, the total fee negotiated for Phase II work is \$123,400. The scope of work for Phase II includes:

- Prepare architectural and structural drawings, and construction documents (plan, specifications, hazardous materials abatement plan) for the Board-selected Long-Term Repair Option.
- Assist in securing permits from Santa Clara County.
- Attend and support the construction bid walks and pre-construction meeting.
- Respond to contractor Requests for Information (RFIs).
- Review and respond to contractor submittals.
- Perform project site inspections and prepare reports.
- Prepare as-built drawings

The following table summarizes the tasks and associated fees.

Phase I – Original Base Contract	\$107,700
Phase I – Spent-to-date Subtotal	(74,275)
Original Base Contract Savings (amount remaining)	33,425
Phase II Fee – Construction Plans and Construction Administration	123,400
Less Original Base Contract Savings	(33,425)
Proposed Amendment to Base Contract Amount	\$89,975
Remaining Contract Contingency (approx. 13% of Phase II Fee)	\$16,155
TOTAL PROPOSED NOT-TO-EXCEED CONTRACT	\$213,830

# FISCAL IMPACT

The FY2019-20 budget includes \$223,750 for the Mount Umunhum - Radar Tower Project VP23-001. There are sufficient funds in the project budget to cover the recommended action and expenditures. Funding for the construction work scheduled next fiscal year will be proposed as part of the upcoming annual Budget and Action Plan process.

Mt. Umunhum - Radar Tower VP23-001	Prior Year Actuals	FY2019-20 Adopted	FY2020-21 Projected	Estimated Future Years	TOTAL
Budget	\$460,765	\$223,750	\$2,488,000	\$0	\$3,172,515
Spent-to-Date (as of 01/15/2020):	(\$460,765)	(\$41,399)	\$0	\$0	(\$502,164)
Encumbrances:	\$0	(\$33,426)	\$0	\$0	(\$33,426)
Wiss, Janney, Elstner Associates, Inc. Contract Phase II:	\$0	(\$77,600)	(\$45,800)	\$0	(\$123,400)
Remaining Contract Contingency:	\$0	\$0	(\$16,155)	\$0	(\$16,155)
Budget Remaining (Proposed):	\$0	\$71,325	\$2,426,045	\$0	\$2,497,370

Staff have investigated grant funding opportunities for the Repair Project. Local grant opportunities to cover portions of the construction costs may be available through the Santa Clara County Historical Heritage Grant Program. However, to be eligible for this grant program, the radar tower must be designated as a historic landmark, which requires owner consent. The radar tower is currently listed on the County's Heritage Resource Inventory, but not designated as a landmark. Landmark status may not be desirable in this instance given that this designation triggers the requirement to have all future minor repairs and alterations reviewed and approved by the County Historical Heritage Commission through a small project review process - per Santa Clara County Ordinance Section C17-17). This requirement does not currently apply to the radar tower as structures listed on the inventory are not subject to this requirement. As such, landmark status for the radar tower would limit the District's ability to conduct future minor repairs to the structure and result in a costly review process and potentially additional conditions of approval to carry out such minor repairs.

Other grant opportunities at the State and National level are limited and very competitive with a low likelihood of award due to the large and competitive application pool. A significant investment in limited District staff time and resources would be required to apply for and administer these grants. The Mount Umunhum Conservancy, a nonprofit organization dedicated to preserving the radar tower, has expressed support for the approved long-term repairs. District staff will discuss funding opportunities with the Conservancy.

The recommended action is not funded by Measure AA.

# **BOARD COMMITTEE REVIEW**

Given the level of Board interest, this item is coming to the full Board for consideration. The Board approved the project goals and scope of work for the Repair Project at the September 12, 2018 regular meeting and authorized the General Manger to release a Request for Proposals to complete the assessment (R-18-105). The Board authorized the General Manager to enter into contract with WJE at the January 23, 2019 regular meeting to prepare a Basis of Design (R-19-09). The Board selected the Long-Term Repairs Option at the November 20, 2019 Special Meeting R-19-155).

# **PUBLIC NOTICE**

Public notice was provided as required by the Brown Act. Notices were also sent to interested parties on the Mount Umunhum mailing list.

## CEQA COMPLIANCE

On August 30, 2010 the Board approved in the Initial Study/Mitigated Negative Declaration (IS/MND) for the Almaden AFS Structure Abatement Project, which included an evaluation of potential repairs and work on the exterior of the radar tower (R-10-102). In addition, on October 17, 2012 the Board adopted the Mount Umunhum Environmental Restoration and Public Access Project Environmental Impact Report, which included an analysis of the Retain and Seal option for the radar tower (R-12-104). The assessment and repair of the radar tower is consistent with the project evaluated in the existing environmental review. No new significant environmental effects or substantial increase in the severity of previously identified effects would result from this project beyond what was analyzed in the existing CEQA documents.

## NEXT STEPS

Pending Board approval, the General Manager will authorize WJE's contract amendment to develop construction documents for the Board-selected Long-Term Repairs Option. Future Board review and input are anticipated to occur as follows:

• Board award of a construction contract for the repair work – Summer/Fall 2020

Attachment:

- 1. Mount Umunhum Radar Tower Assessment Project Basis of Design WJE Nov 2019
- 2. Recent Project History

Responsible Department Head:

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Prepared by:

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# MOUNT UMUNHUM RADAR TOWER Condition Assessment and Recommendations

Sierra Azul Open Space Preserve Santa Cruz Mountains, California



**Final Report** November 13, 2019 WJE No. 2018.0484



Prepared for: Midpeninsula Regional Open Space District 330 Distel Circle Los Altos, California 94022

Prepared by: Wiss, Janney, Elstner Associates, Inc. 2000 Powell Street, Suite 1650 Emeryville, California 94608 510.428.2907 tel | 510.428.0456 fax



# MOUNT UMUNHUM RADAR TOWER Condition Assessment and Recommendations

Sierra Azul Open Space Preserve Santa Cruz Mountains, California

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# MOUNT UMUNHUM RADAR TOWER Condition Assessment and Recommendations

Sierra Azul Open Space Preserve Santa Cruz Mountains, California

## **EXECUTIVE SUMMARY**

Wiss, Janney, Elstner Associates, Inc. (WJE) has been engaged by the Midpeninsula Regional Open Space District (District) to perform a structural and waterproofing evaluation of the former radar tower at the top of Mount Umunhum. This report provides our preliminary assessment of the building's present condition and recommendations to better seal the building against water intrusion and mitigate ongoing deterioration of the concrete structure.

In its current condition, several potential safety hazards exist at the radar tower, including loose concrete on the exterior walls and roof. Mitigation of some of these hazards is discussed further in this report and recommended to be completed prior to allowing public access at the area immediately adjacent to the radar tower. For the strategy of retain and seal that has been selected by the District, additional measures should be implemented to preserve the existing building and minimize future deterioration. In addition to the safety hazards, remediation of the water intrusion into the building through the walls and roof is recommended to mitigate further deterioration and to prevent the growth of mold or other materials that may propagate in the presence of moisture and stagnant air.

## BACKGROUND

The Mount Umunhum radar tower is an 85-foot tall concrete building constructed from 1958 to 1962 as part of the Almaden Air Force Station. The radar tower building supported a large radar antenna (Figure 1) at the roof and operated from 1962 until 1980 when it was decommissioned and the radar antenna at the top of the building was removed. The building is located at the top of Mount Umunhum (Figure 2 through Figure 4) at an elevation of approximately 3,490 feet above sea level in what is now part of the District's Sierra Azul Open Space Preserve. The site is exposed to harsh weather conditions including high wind speeds and freezing temperatures in the winter.

The building has received limited maintenance since its decommissioning in 1980. The District acquired the site in 1986, but the radar tower has remained closed to the public due to hazardous materials and contamination at the former Air Force Station. Federal funding allowed for the removal of fuel-based hazardous materials in the mid-1990s. A survey of hazardous building materials performed by IHI Environmental in 2010 identified a number of asbestos- and lead-containing materials within the building which were recommended to be properly remediated. Additional Federal funding allowed for the removal of loose and peeling lead-containing paint on the exterior and interior of the radar tower in 2011. Limited abatement of hazardous materials was performed as recommended in 2011 by the Army Corps of Engineers and IHI Environmental provided monitoring of the abatement, however asbestos and lead hazards remain at the building. All of the Air Force Station buildings and structures except for the radar tower were removed in 2013.

The radar tower was listed on the Santa Clara County Heritage Resource Inventory in 2016 and the District Board of Directors (District Board) approved a "Retain and Seal" option for the long-term treatment of the radar tower, intended to reduce the incidence of water intrusion into the building in an effort to increase its longevity.





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In July of 2016 an *Interim Radar Tower Repair Project* was completed, including short-term repairs focused on sealing openings to limit water and wildlife intrusion, removing unstable roof debris, and ground-level concrete repair. Landscaping and drainage improvements were also installed in 2017, including installation of the pathway around the tower. The Mount Umunhum Environmental Restoration and Public Access Project opened both the east and west mountain summits to public access, including around the radar tower site, in September of 2017.

In November of 2017, flakes of paint and concrete debris found on the ground adjacent to the radar tower tested positive for lead and asbestos, respectively. The area surrounding the radar tower, including the pathway to the east summit, was fenced off to prevent public access due to potential health and safety concerns. In June of 2018, all remaining paint was removed from the exterior of the building and further testing identified asbestos in some exterior concrete patching materials.

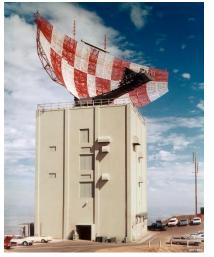


Figure 1. Historic photo of Mount Umunhum radar tower facing east. Photo courtesy of Midpeninsula Open Space District.



*Figure 3. Aerial photo of the radar tower, facing north.* 

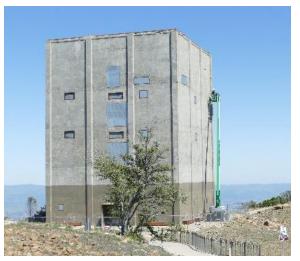


Figure 2. Recent exterior view of the radar tower facing east.



Figure 4. Photo of the radar tower, facing west.



Since the complete removal of the exterior paint, concrete debris as wide as 1.5 inches has been observed on the ground around the base of the building and the area surrounding the building remains closed to the public. A survey of the exterior concrete walls was performed by ZFA Structural Engineers in July of 2018 to determine the extent of deterioration and provide recommendations on improving the future performance of the concrete walls. Several alternative methods for addressing the concrete deterioration were presented.

The District engaged WJE to further evaluate the radar tower and provide repair recommendations that meet the following District Board approved criteria:

- Supports the Board-approved "Retain and Seal" option
- Ensures public and worker safety around the radar tower
- Avoids future contamination concerns
- Reduces (or eliminates) future need to enter building
- Protects workers if/when ingress is needed
- Avoids wildlife trappings and other resource impacts
- Reopens a visitor pathway to the east summit

At the request of the District, the Discussion and Basis of Design sections are presented first. The Basis of Design provides recommendations to meet the District Board approved criteria. The characteristics of the building and site observations are presented in the Building Description and Site Observation sections that follow the Discussion and Basis of Design sections.

#### DISCUSSION

There is widespread evidence of water leakage through the roof, concrete walls, and at the wall openings, and previous repairs at the exterior and staining at the interior suggest long-term leakage at these areas. Although some past repairs to the exterior walls and roof have been completed, the current poor condition of the exterior surfaces of the concrete and corroded steel surfaces at the walls and roof are not suitable as a substrate for paint, fluid-applied, or adhered waterproofing membranes for mitigating moisture intrusion, and would require significant surface repair and patching for use as a sound substrate for an adhered membrane.

## **Deferred Repairs**

In its current state, the radar tower will continue to deteriorate if no efforts are made to remediate the observed conditions at the building enclosure. Continued deterioration of the walls and roof will allow additional water into the building which will cause further deterioration, likely increasing the rate of deterioration over time. The increased rate of deterioration can significantly increase the amount of repairs that will be necessary in the future. Some of the potential impacts of delaying repairs are discussed below.

**Collapse Potential** - There is very little chance of large-scale collapse of the radar tower in the next one hundred years even if no action is taken to address the current conditions. The structure is very robust as an unused structure that was designed to support heavy equipment that is no longer present. The building is also constructed of reinforced concrete, which is somewhat tolerant of moderate exposure to moisture, so the structural framing is anticipated to continue to support the floors and roof even with further deterioration.

*Risks to the Public* - The existing fencing around the radar tower, if maintained, is expected to provide adequate protection to the public from debris that may fall from the radar tower as the roof and concrete walls continue to deteriorate. Both of these conditions are likely to allow small pieces of concrete or other





debris to fall to the ground around the radar tower. As deterioration of the concrete walls and roof progresses, the size of the debris that may fall from the structure will likely increase, but larger pieces will not be transported as far from the radar tower by the wind as smaller pieces, and therefore will not increase the public hazard. Eventually, corroded steel elements from the roof are likely to become detached due to deterioration and be transported off the roof due to winds.

A covered walkway along the south side should be able to provide adequate protection to the public against falling debris, at least for the immediate future. Eventually, the size of debris that may become dislodged will exceed the capacity of the walkway to provide protection. It should be noted that due to the conditions at the site, a covered walkway would need to be designed as a permanent structure, capable to resisting high winds and it would also need to extend beyond the building to the east and west by at least half of the length of the structure in each direction to protect against falling debris that may be wind-blown.

Hazardous Materials Contamination - Most of the identified hazardous materials are located within the interior of the radar tower. Since the openings in the exterior walls are minimal and the doors for access into the building are opened infrequently, the potential for release of hazardous materials to the exterior of the radar tower will be negligible in the long term even with no repairs or abatement. The asbestoscontaining concrete materials on the exterior walls however will gradually deteriorate and could produce airborne particles, particularly from the radar pedestal on the roof that is already significantly deteriorated. Small amounts of this concrete have likely been released already, and the amount will increase over time as the concrete continues to deteriorate. All concrete debris from the building will need to be treated as potentially containing asbestos and will need to be cleaned up on a regular basis using appropriate handling and disposal procedures. The falling debris would need to be cleaned up by personnel that have asbestos awareness training, with no other precautions. The frequency of the cleanup is hard to judge, but would probably be required at least once per year to start and would likely need to be more frequent as the deterioration of the concrete accelerates. SCA Environmental does not believe that the minor amounts of falling concrete debris would pose a threat of exposure to the public. Some materials that are part of the roofing system are also asbestos-containing and will also eventually deteriorate and become detached, creating a potential for release.

**Risks to Maintenance Personnel** - Existing plywood covers at floor openings in areas that may be exposed to water that leaks through the walls or wall openings are the items most likely to deteriorate into an unsafe condition in the near future if repairs to the building enclosure are delayed. Decay of the plywood could weaken these to a condition where they are unable to support the weight of a person that may inadvertently step on them.

#### Walls

Recent complete removal of the exterior paint may have increased what was likely pre-existing, long-term leakage condition through the concrete wall openings, joints, cracks, and voids due to poor concrete consolidation. Removal of the existing coating has uncovered some of the underlying areas of poorly consolidated concrete and some of the exposed reinforcing steel that has since started to corrode.

The areas of most extensive voids and loose concrete due to poor consolidation were observed to occur in the portions of the walls directly above the third through fifth floor slabs. The surface voids and loose concrete are in large part due to poor construction quality of the original concrete that resulted in areas of incomplete concrete consolidation. Based on our observations, the concrete for the walls was cast on top of the floor slabs one story level at a time. This led to the formwork for the walls above the third floor being over 15 feet high. Concrete placed into tall, narrow formwork can become segregated as it drops into the formwork, meaning that the cement paste and aggregate do not stay adequately mixed without the



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application of proper vibration. The voiding observed shows that the larger aggregate particles have only marginal amounts of cement paste surrounding them, which is consistent with the segregation that would occur with tall formwork if the concrete was not properly vibrated during placement. Although acoustic hammer sounding of the walls identified localized delaminations, many areas of visually rough and poorly consolidated concrete were otherwise sound and remain well bonded to the wall. These rough surfaces however are not conducive to providing an adequate substrate for the installation of a waterproofing coating.

Repairs are needed to protect the concrete from further deterioration due to corrosion of the embedded reinforcing steel, and to prevent moisture intrusion into the building. The most extensive concrete deterioration occurs at the pilasters, where large surface voids are present and corroded reinforcing steel is exposed, and along the perimeter roof edge beam where it protrudes past the face of the wall below. In addition, crack and spall repairs are needed at various locations at the exterior walls where rebar is exposed or corroding, and significant shallow surface patching of rough, but otherwise sound, concrete would also be required to be repaired to provide a substrate for a coating to resist water penetration through the concrete walls. An alternative approach is to abandon the concrete walls as a substrate for a waterproofing coating and install a mechanically attached cladding system between the pilasters, such as cement plaster (stucco), which is not reliant on a smooth concrete substrate for reliable performance, and would provide a suitable substrate for a waterproofing coating. This alternative would include limited structural repairs to the concrete walls (between pilasters), only where necessary, and would perform more extensive concrete spall and rough surface repairs at the pilasters to prepare the surface for a waterproofing coating. These two approaches are discussed in the Basis of Design section below. Other materials that could be applied to the exterior surfaces of the walls to protect the existing concrete were evaluated such as concrete, metal panels, and polymer fibers; however these other materials would be more costly to install or would significantly alter the exterior appearance.

## Wall Openings

The existing openings in the exterior walls other than two of the ground level doors have been infilled with either concrete or steel plates. Where steel plates have been installed, a small gap exists around the perimeter that allows ventilation and prevents wildlife intrusion into the building, but these gaps also allow water infiltration due to wind-driven rain. Three strategies can be considered for improving the weather resistance of the existing openings; likely a combination of multiple strategies should be selectively applied to the openings across the entire building. First, the openings can be maintained in their current condition while providing a new system to collect and discharge rain water that may enter through the existing steel plate assemblies. Second, the steel plates at openings can be replaced with louvers to allow air movement but reduce water intrusion and prevent wildlife intrusion. Thirdly, select openings could be permanently sealed to prevent water and wildlife intrusion.

Strategies two and three discussed above alter the existing ventilation capacity of the wall openings. The ventilation requirements for the unoccupied building are minimal; however application of these strategies across all of the wall openings should include an evaluation of the ventilation requirements for the entire building. Selection of specific repair strategies at individual openings should be coordinated to maintain adequate passive ventilation in the building for safe access by maintenance personnel, and to minimize water intrusion.

Sheet metal hoods could be added to any of the above strategies at locations that originally had hoods and provide additional weather protection to active louvers. Alternately, there may be cost savings by locating and configuring active louvers in the openings in the most efficient layout to satisfy the ventilation



requirements while reducing the potential for water intrusion. Application of these strategies to the radar tower, as three repair options, are discussed in the Basis of Design section below.

#### Roof

Conditions at the roof present multiple challenges for both interim repairs and installation of a long-term roofing system. In addition to the generally deteriorated, rough, weathered and poorly consolidated concrete surfaces, numerous corroded penetrations, obstructions, and other objects exposed at the roof significantly complicate the surface preparation and geometry of repairs. These combined conditions present significant challenges for the use of fluid-applied and self-adhering waterproofing membranes that might otherwise be an economical interim solution to leakage at the roof until long-term, low-maintenance repairs can be installed. A fluid-applied membrane was previously applied to the wearing slab, suggesting past leakage, but it is heavily deteriorated and is no longer effective.

Based on the District's stated goals to retain and seal the building and to reduce the necessary maintenance, the District should consider whether items protruding from the roof that complicate roofing repairs can be removed. Although the radar tower is listed on the Santa Clara County Heritage Resource Inventory, it is only intended to be viewed by the public from the ground level; not accessed from the interior or viewed from the roof. Removal of these abandoned items would significantly simplify the design of the new roof and therefore would also likely reduce the construction cost and necessary maintenance of the system.

The presence of the existing built-up waterproofing membrane beneath the wearing slab creates another challenge for the design of new roofing systems that are overlaid on top of the wearing slab, especially for adhered membranes, either sheet or fluid-applied. There is likely some amount of moisture trapped within the assembly due to natural exposure and ongoing leakage. This entrapped moisture could weaken the adhesion of new adhered membranes, or could cause condensation, corrosion or biological growth if the roof design does not adequately address this condition. Due to the significant amount of surface preparation that would be required and the possibility for moisture to be trapped within the existing roof assembly, the installation of a new fluid-applied membrane was not considered as a viable option to meet the project's stated goals. These conditions and limitations apply to any roofing systems that relies on adhesion to the existing concrete substrates, including modified bitumen membranes, single-ply membranes and sprayapplied foam roofing. While mechanically attaching some of these systems directly to the existing concrete substrates may be possible, achieving a suitable system would be difficult, likely requiring installation of a cover or substrate board, predrilling fasteners, and removing the numerous obstructions at the roof to be flush with the roof deck. These factors led to considering methods of abandoning the existing concrete as a substrate and providing a new substrate suitable for new roofing, as described in the next section. An additional advantage of this approach is the ability to envelop the smaller obstructions within the new assembly, reducing the need to remove them to be completely flush with the roof deck.

The District has asked WJE to provide additional information about the possible use of spray foam roofing systems, such as Durafoam, for the Radar Tower roof. As discussed above, the poor condition of the existing concrete substrates, and the presence of the existing built-up waterproofing membrane beneath the wearing slab are not conducive to the use of any adhered roofing systems, including spray foam roofing, without significant surface preparation or substrate construction. If the concrete surface was sufficiently repaired and prepared, or a new substrate was constructed that is suitable to receive an adhered roofing system, and it is determined that moisture that may be trapped within the existing roofing assembly will not degrade the bond of an adhered system, there are other adhered sheet, or fluid-applied roofing membranes that offer significant advantages over spray foam roofing, especially regarding overall durability and required maintenance, which are important priorities of the District for this project.



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Some additional drawbacks of using a spray foam roofing system at the Radar Tower include:

- The up-front material costs can be high, and installers often attempt to cut corners,
- They require a highly skilled contractor to install correctly,
- The market share for spray foam roofing is small, and skilled contractors are limited,
- There are limitations on the weather conditions (including wind speed) under which they can be installed,
- Any moisture trapped in the substrate, or contaminants on the substrate will affect the bond of the roofing,
- They are vulnerable to puncture or damage from foot traffic, hail, wind-driven debris and birds,
- On low-slope applications, their irregular surface can inhibit drainage and cause ponding and premature deterioration,
- It is recommended that they be inspected twice annually due to their limited durability,
- It is recommended that they be inspected for damage after high wind events,
- They require recoating every 8 to 10 years.

Due to the significant exposure to high wind, the new roof design should provide adequate attachment for all components of the roof system.

The existing roof lacks a functioning drainage system since the existing drainage plumbing was reportedly disconnected and sealed with grout. The Interim Repair Project Construction Drawings initially included reestablishment of roof drainage; however the notes and details showing this work are crossed out and were apparently removed from the project. The work would have included cutting the roof drainage pipes at ground level and routing them through the south wall onto a concrete splash block at grade.

Further evaluation will be required to determine the feasibility of reestablishing drainage through the existing roof drains and plumbing for either short-term or long-term use. It could not be determined if the roof drainage plumbing is also sealed just below the roof drains to prevent the plumbing from completely filling with water.

Any repair strategy for the roof system will require occasional access to the roof for inspection and maintenance, annually at a minimum. The current roof system does not have a fall protection or guardrail system that is compliant with Cal/OSHA regulations since the original perimeter railing system was removed. The current procedure for roof access requires trained personnel to provide adequate rigging to attach lifelines. As the roof continues to deteriorate, the availability of adequate, safe attachment points for lifelines will diminish. An appropriate system should be installed at the roof level to allow for safe access to the roof for periodic inspection and maintenance. One option would be the installation of new perimeter guardrails, which would likely require strengthening of the existing steel brackets that supported the original rails. The replacement guardrail should utilize more durable materials than the original wood guardrail. Alternately, since access to the roof is expected to be minimal, fall protection anchors can be installed on the roof around the concrete pedestal and a small guardrail system can be installed around the roof hatch to protect people from the fall hazard as they travel from the roof hatch at the edge of the roof to the central concrete pedestal to attach to the fall protection anchors.

# **Potential Interior Hazards**

Several hazardous conditions exist at the interior of the radar tower, including the presence of hazardous materials, missing or damaged guardrails, openings in the existing floors, and the presence of abandoned equipment and debris. The District currently has restrictions in place for accessing the building interior, and these restrictions seem appropriate given the extent of the existing hazards. Since frequent interior access or occupancy is not anticipated, short-term remedial measures should address safety for periodic



maintenance use and implementing the interior and roof repairs. Fall hazards at the interior are limited to the stairs and the openings in the floors. Comprehensive remediation of the hazardous materials and removal of debris would be recommended as part of a long-term program.

## **Exterior Doors**

Due to the remote location of the radar tower, the public use of the site, and the unoccupied condition of the building, site and building security and protection against vandalism are important project goals. In addition to re-application of an anti-graffiti coating along the base of the exterior walls after they are repaired, further improvements to the security of the entry doors may include additional heavy-duty locks added to the existing doors.

#### Site

The wood benches around the building are located very close to the exterior walls, which will make it difficult to install any concrete patching, coating, or other finishes on the portion of the wall obstructed by the benches. Since this occurs at the base of the walls, the omission of a new coating or finish at these location would not result in a condition where concrete spalling would cause a safety hazard, and would be unlikely to cause significant water intrusion.

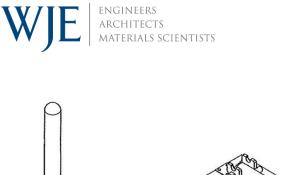
The existing removable bollards between the parking area and the radar tower are susceptible to sand and other debris falling into the gap between the bollard and the sleeve embedded in the concrete, making the bollards difficult to remove. If the existing bollards are to remain, we recommend that on a monthly basis the bollards be removed, the surfaces of the bollards be wiped to remove adhered debris, and the inside of the sleeves be cleaned of debris. We recommend replacing the removable bollards with a different style of removable bollard that will be easier to operate and will require less maintenance.

# **BASIS OF DESIGN**

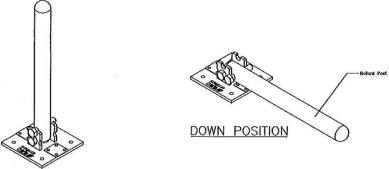
The intent of the following proposed repairs is to follow the District's decision to retain and seal the radar tower building. The recommended strategies address the conditions identified during our evaluation, which are described in detail in the later Building Description and Site Observations sections. The proposed repairs are intended to allow safe access by the public around the exterior of the building, mitigate water intrusion into the building, and provide safe occasional access into the building for maintenance purposes. Since the building will receive only periodic maintenance, the proposed repairs also intend to minimize ongoing maintenance needs. A preliminary cost estimate for the base repairs and various options was provided by our cost estimating sub consultant and is included in Appendix B.

# **Site Conditions**

Replace the two removable bollards with a bollard system that is tolerant of exposure to sand and debris. A foldable bollard system, similar to that used along the roadway to the peak of Mount Umunhum (Figure 5) would appear to be easier to operate and would be less likely to become jammed with debris. Replacement bollards would be expected to last at least 30 years.



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UP POSITION

Figure 5. Foldable bollard used on Mt. Umunhum Road.

## **Exterior Doors**

## **Option 1 - Maintain in Existing Configuration**

The doors can be maintained in their existing configuration. We recommend performing the following repairs at the two remaining doors to improve their weather resistance. Given that the existing doors appear to be stainless steel, if they are coated with a high performance coating, they would be expected to last more than 50 years, with overcoating of the high performance coating after 20 years, and removal and replacement of the coating after 40 years.

This repair option includes:

- Install weather stripping at the doors.
  - Install gaskets around the door perimeter (all four sides).
  - Install a door sweep at the bottom edge of the door.
  - Modify the threshold to engage the gasket/sweep at the bottom edge of the door.
- Provide new heavy duty door locks.
- Prepare and paint the steel doors to protect them from corrosion.

Advantages	Disadvantages
Lower cost	• Performance improvements may be limited

#### **Option 2 - Replace with New Doors**

To improve the functionality of the doors, the existing doors can be removed and replaced with new security doors. Installation of the new doors will include a new frame for the doors including thresholds and weather seals around the perimeter. The new doors should include internal locks and panic hardware on the interior. Replacement doors will likely be non-stainless steel, but would also be coated with a high performance coating. Replacement doors would be expected to last at least 50 years, with overcoating of the high performance coating after 20 years, and removal and replacement of the high performance coating after 40 years.

Advantages	Disadvantages
Improved weather resistance	Higher cost
• Improved security and safety	



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#### **Potential Interior Hazards**

Short-term repairs at the interior focus on providing safe access to the floors of the building for occasional maintenance access into the building, including replacing missing and damaged handrails and addressing existing holes in the floors. Long-term repairs are intended to eliminate hazardous materials and implement additional measures as recommended for securing the radar tower. Access to the roof for maintenance and repair is discussed in the Roof section above.

#### Short-Term Repairs

- Review the existing plywood covers at floor openings to identify and repair any that may be damaged due to exposure to moisture.
- Cover holes in fifth floor mezzanine floor with steel plates, or restrict access to the mezzanine.
- Replace missing or damaged stair guardrails.

#### Long-Term Repairs

- Remove all non-essential equipment and debris from the interior.
- Replace plywood covers at floor openings with steel plate covers.
- Abate hazardous materials, including asbestos and lead-based paint.

#### **Concrete Wall Repair**

The existing walls have numerous areas with surface voids due to poor consolidation, loose concrete, and exposed reinforcing steel. Some of these locations are susceptible to allowing small pieces of concrete or aggregate to become dislodged and fall, creating a potential safety hazard. The surface voids expose reinforcing steel, which has started to corrode. This will lead to more concrete spalling or becoming dislodged as the corrosion progresses. The proposed repair design includes a base level of repairs to address the most significant areas of deterioration at the pilasters, perimeter roof edge beam at the top of the walls, and exterior wall surfaces. Additional surface repairs or over-cladding of the exterior wall surfaces between the pilasters and at the perimeter roof edge beam are necessary to mitigate the falling hazard and to provide an adequate substrate for application of a waterproof elastomeric wall coating. We considered several options for over cladding of the concrete walls including additional concrete, metal panels, and adhered fiberglass sheets. Each of these options were judged to have a significant negative impact on the appearance of the radar tower and would likely be more expensive than the options discussed below. The wall coating will also provide a protective layer over the asbestos-containing patching material in the concrete walls and mortar for the concrete block wind screens to limit further deterioration and damage due to water intrusion.

#### **Base Wall Repairs**

At a minimum, we recommend performing the following repairs at the walls to address the most significant areas of distress using common concrete repair procedures. Schematic details for these repairs are provided in Figure 6 through Figure 8. These repairs will be needed regardless of which other repair strategies for the remaining portions of the wall are selected. The base exterior concrete repairs are intended to address the conditions of significant concrete deterioration including the pilasters that provide a significant portion of the load-carrying capacity of the walls. The base repairs are likely to allow the structure to remain standing without collapse for at least one hundred years. Implementing only the base repairs, without Options 1 or 2, however does not allow for the installation of an elastomeric coating on the walls. The lack of coating on the walls will lead to continued moisture penetration into the building, resulting in standing water and deterioration of the floors and interior. WJE recommends the following base wall repairs:

• Repair large cracks, voids, spalls, and delaminations at pilasters, at the perimeter roof edge beam along the top of the walls, and at portions of the exterior walls with deep voids. These repairs should



include all locations where the reinforcing steel is exposed and any other location where the depth of the spall or void exceeds 1 inch.

- Remove loose or delaminated concrete and sawcut the perimeter of the repair area.
- Chip out the concrete to allow access around the exposed reinforcing steel.
- Clean the corrosion and apply a protective coating to the exposed steel.
- Supplement severely corroded or missing reinforcing steel with new reinforcing steel.
- Patch the concrete spalls using an appropriate concrete repair mortar.
- Coat concrete walls and pilasters with an elastomeric coating.
- Re-apply an anti-graffiti coating on the lower portion of the walls, similar to what currently exists at the building.

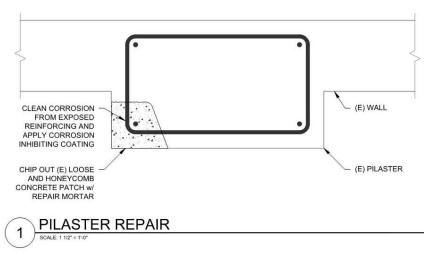


Figure 6. Base concrete pilaster repair.

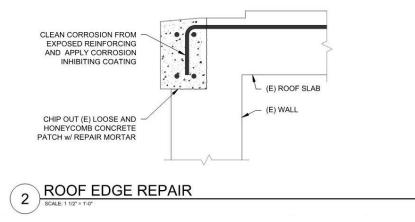


Figure 7. Base perimeter roof edge beam repair.

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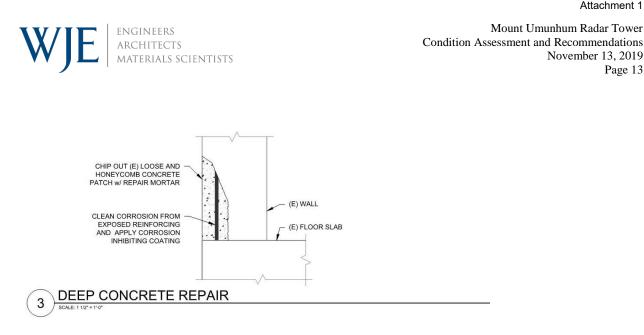


Figure 8. Base deep concrete void or spall repair.

## **Option 1 - Patch and Coat**

This option repairs the surface of the exterior concrete walls between the pilasters to be suitable to receive an elastomeric waterproofing coating. Selection of an appropriate repair material for this option is critical since the depth of the patches will be very thin in some locations. This repair option maintains the original concrete surface profile of the entire building. The elastomeric coating would be expected to be overcoated every 5 to 7 years and removed and replaced after about 20 years. The concrete patching would last 30 to 40 years as long as the elastomeric coating is maintained.

This repair option includes:

- Repair shallow rough and poorly consolidated concrete at walls.
  - Remove loose concrete and roughen surface to receive concrete repair mortar. 0
  - $\cap$ Patch concrete for smooth, well-bonded finish.

Advantages	Disadvantages
<ul> <li>Maintains the original concrete surface</li> <li>Creates a surface that can receive a waterproofing coating</li> </ul>	<ul> <li>Significant surface patching and preparation will be required</li> <li>Potential for additional concrete spalling in areas not patched</li> </ul>

## **Option 2 - Stucco Cladding Between Pilasters**

This option allows the existing rough surface to remain and installs mechanically attached 3-coat cement plaster (stucco) system (thickness of about 1 inch) on the walls between the concrete pilasters as a substrate for an elastomeric waterproofing coating (Figure 9). The finish of the stucco surface should be smooth to match the texture of a formed concrete surface; however crack control joints needed in the stucco and sealant joints needed between the stucco and the pilasters will be visible deviations from the current appearance. The elastomeric coating would be expected to be overcoated every 5 to 7 years and removed and replaced after about 20 years. The stucco cladding would last at least 40 years as long as the elastomeric coating is maintained, and the sealant would need to be replaced after about 20 years.

This repair option includes:

Install mechanically fastened metal lath and 3-coat stucco on the exterior walls between pilasters, and below the perimeter roof edge beam.



• Install horizontal control joints at the bottom of each floor slab level (top of each story wall), and intermediate horizontal control joints at the mid-height of the fourth and fifth floor walls.

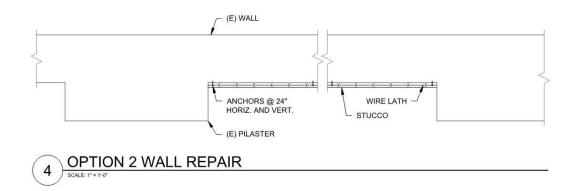


Figure 9. Wall repair option using a mechanically attached stucco system applied to the walls.

Advantages	Disadvantages			
<ul> <li>Does not require extensive surface patching at concrete walls</li> <li>Provides a uniform surface for coating</li> <li>Provides a secondary layer of protection to limit the amount of water than can penetrate to the reinforcing steel and cause corrosion</li> </ul>	<ul> <li>Requires installation of joints to control stucco cracking</li> <li>More costly</li> </ul>			

# **Roof Repairs**

Short-term or interim repairs at the roof could be implemented to mitigate some of the water intrusion but unfortunately the effectiveness of these repairs are limited by the poor condition and complex configuration of concrete and steel substrates. Targeted maintenance repairs and water management strategies are described as a short-term or interim measures to reduce or manage ongoing leakage until a long-term roofing system can be designed and installed. Given the District's stated goal to reduce ongoing maintenance, and the various challenges described in the Roof Discussion section above, we feel that there are two primary long-term roofing strategies, discussed in detail below. Base repairs will be needed in addition to implementing either the short-term or long-term strategies, including the installation of a fall protection system to allow periodic inspection, maintenance and repairs.

#### **Base Roof Repairs**

At a minimum, we recommend performing the following repairs at the roof regardless of which other repair strategies are elected:

- Reestablish roof drainage. There are three possible approaches to reestablishing roof drainage:
  - **Option 1**: Reroute the existing roof drainage plumbing at the first floor to discharge at a splash block or other appropriate site drainage infrastructure to the southeast of the building. Further evaluation will be required to confirm that the existing drains and plumbing are in suitable condition, and to determine what anticipated service life may remain in the existing drainage plumbing system. This evaluation may determine that replacement of the roof drains is necessary, but the existing plumbing can be retained and

repaired, that this is only a short-term strategy and new plumbing should be installed when a new roof is installed at the building, or possibly that the existing plumbing cannot be reused at all. The service life for reusing the existing plumbing will depend on the findings of a more detailed evaluation of the condition of the existing plumbing.

Advantages	Disadvantages
<ul> <li>Minimal scope can be implemented at any time (if feasible)</li> <li>Significantly less expensive than other options</li> </ul>	<ul> <li>Requires evaluation of existing plumbing for reuse</li> <li>May be determined to be a short-term solution</li> <li>May not be feasible</li> <li>Plumbing leakage would introduce water into the building</li> </ul>

• **Option 2**: Replace the four existing roof drains and pipes with new plumbing. The general configuration would match the existing layout, with a similar discharge at the southeast corner of the building onto a splash block or existing site drainage infrastructure. New plumbing would be expected to last more than 50 years with periodic maintenance.

Advantages	Disadvantages
• Significantly extends the expected service life	<ul> <li>Likely the most expensive of the three options</li> <li>Plumbing leakage would introduce water into the building</li> </ul>

• **Option 3**: An alternate roof drainage strategy could utilize gutters and external downspouts. Gutters and downspouts may be designed and installed in a way to minimize the aesthetic impact on the structure. The use of an externally-plumbed drainage system would be dependent on the new roof design providing new slope towards the roof perimeter. Gutters and external downspouts would be expected to last about 30 years, with maintenance and repair at 10 year intervals.

Advar	ntages				Disady	vantages					
•	Likely	less	expensive	than	•	Visible	at	the	exterior	of	the
	Option 2	2	_			structure	e				

- Remove the aluminum sheet shielding from the underside of the roof slab to observe the condition of the slab for locations of water intrusion and possible structural damage. Inspect the structural slab soffit and repair damage.
- The vertical surfaces (walls) of the concrete pedestal should be patched and coated with elastomeric coating.

#### Short-Term Targeted Maintenance Repairs and Water Management

A strategy that may be economical in the short-term is to monitor and manage water intrusion, performing targeted maintenance repairs where leakage is specifically occurring. This strategy is unlikely to eliminate leakage or prevent further deterioration of exposed materials at the roof, and the steel framing beneath areas where leakage persists will continue to deteriorate. This strategy will also require more maintenance activity by District staff, which is inconsistent with the project's stated long-term goals, but this strategy is offered



as an achievable short-term or interim goal. Maintenance at the roof should be performed annually, at a minimum with any strategy, and possibly more frequently with this strategy. This strategy also assumes that the surface membrane on top of the wearing slab is the membrane to be maintained since the original roofing membrane is concealed beneath the wearing slab and cannot be maintained. Some of the existing sealants used beneath the surface membrane contain asbestos and will need to be contained to prevent deterioration.

This repair strategy includes:

- Review leakage at the underside of the roof slab (after removal of the aluminum sheet shielding) to diagnose specific locations where water may be entering.
- Observe and document leakage through the roof during (and after) rainstorms. Maintain a leak log to track leakage and target areas where active leakage is originating.
- Perform targeted maintenance above areas where leakage is occurring with chemical-injection, sealant, sheet membranes, sheet metal, and possibly fluid-applied membranes, as appropriate. Performance of repairs may be limited by poor surface conditions and limited surface preparation. Follow-up repairs should be anticipated.
- Reseal expansion joints to encourage surface drainage and limit water penetration.
- Seal open gaps and joints around penetrations, pitch pockets, hatches and other rooftop items across the entire roof.

#### Long-Term Roofing System

For a long-term strategy, the existing membranes should be abandoned or removed, and a new waterproofing membrane should be installed. Two strategies for a new membrane are described below.

#### **Option 1: Single-Ply Roofing Membrane over New Roof Framing.**

This option abandons the concrete and steel surfaces as waterproofing membrane substrates and installs new light gauge metal framing and exterior sheathing with a single-ply membrane that is adhered, or both mechanically fastened and adhered to the sheathing. Single-ply membranes are durable and generally require minimal maintenance. As the roof is fully exposed to the sun, and will have minimal foot traffic, a membrane that is especially resistant to ultraviolet radiation should be selected. A durable single-ply roof membrane would be expected to last about 20 to 30 years with annual inspections and maintenance. Although the new roof framing alters the roofing assembly, the additional framing will not significantly impact the appearance of the roof from the ground.

This repair strategy includes:

- Remove rooftop penetrations and obstructions where possible.
  - Detach items that can be removed without damage.
  - Cut penetrating items close to flush with the top surface of the wearing slab or concrete pedestal.
- Coordinate the height of new and existing rooftop items to remain with the increased height of the new roof deck.
  - Remove the roof hatch and reconfigure roof access with a new roof hatch to accommodate the new (increased) height of the roof deck.
  - Coordinate location of fall protection anchors and guardrail height with height of new roof deck.
- Install light gauge metal framing with exterior sheathing to create a new sloped roof deck across the entire roof, including above the concrete pedestal. Ventilate the new cavity between the sheathing and wearing slab, or fill the cavity with closed-cell spray foam insulation for an unvented system.



- Conceal remaining rooftop penetrations and obstructions within the new roof deck assembly, or with box-outs where the penetration or obstruction is taller than the level of the new roof deck (sheathing). Protect existing materials with a separation layer, and fill cavity with closed-cell spray foam insulation to minimize the risk of condensation within the assembly.
- Slope new roofing to the location of existing roof drains and integrate with new drains. Alternately, the roof deck could be sloped towards the roof perimeter to utilize gutters and downspouts.

Advantages	Disadvantages
<ul> <li>Abandons deteriorated concrete and steel surfaces</li> <li>Simplifies geometry of roofing details</li> <li>Tolerant of moisture that may be trapped within the existing roof assembly</li> <li>Likely less expensive than Option 2</li> </ul>	<ul> <li>Roofing membrane is fully exposed to environment and will deteriorate due to UV exposure</li> <li>Requires removal of rooftop penetrations</li> </ul>

#### **Option 2: Buried Roofing Membrane**

This option would remove and replace the concrete wearing slab and original built-up roofing membrane with a hot rubberized asphalt (HRA) membrane with a new concrete wearing slab for protection. Additionally, a similar HRA membrane with concrete wearing slab would be installed on top of the concrete pedestal after removal of obstructions and surface patching and preparation.

This strategy is likely significantly more expensive than the single-ply membrane in Option 1, but offers greater protection of the roofing membranes from solar exposure with new wearing slabs. Some of the existing materials that will be removed contain asbestos, so special procedures will be needed for safe removal and disposal of the debris. The condition of the underlying structural slab is not known and will need to be investigated to confirm the viability of this approach after removal of the aluminum sheets on the underside of the roof and removal of the existing built-up membrane.

The strategy outlined below includes installation of an HRA membrane on top of the concrete pedestal which will require significant concrete repair work. It would also be possible to install a single-ply membrane on new light gauge metal framing and exterior sheathing, similar to that described above in Option 1, at the concrete pedestal as a cost saving measure. A buried HRA membrane would be expected to last about 30 to 40 years with annual inspections and minimal maintenance.

This repair strategy includes:

- Remove concrete wearing slab and underlying roofing assembly to expose the structural concrete slab.
  - Inspect the top of the structural slab and repair damage.
- Repair rough, delaminated and spalled concrete on both horizontal and vertical surfaces at the concrete pedestal.
- Remove rooftop penetrations and obstructions where possible.
  - Detach items that can be removed without damage.
  - Cut penetrating items flush with the top surface of the structural slab or concrete pedestal.
- Prepare remaining rooftop penetrations and obstructions for integration with HRA membrane by sand blasting steel.
- Install HRA membrane with new concrete wearing slab on horizontal roof surfaces.
  - Integrate HRA membrane with new roof drains in original locations.

• Remove corrosion from steel that is exposed above the roof and coat with a high performance coating system.

Advantages	Disadvantages
• HRA is durable, and will be protected by a topping slab	<ul> <li>Condition of structural roof slab will require investigation</li> <li>Significant concrete repair will be required at the concrete pedestal in order to use HRA there</li> <li>Removal and replacement of the topping slab adds significant expense</li> </ul>

# **Fall Protection**

As part of the base roof repairs, install fall protection measures to prevent the exposure of maintenance workers on the roof to fall hazards. This can be accomplished in several ways:

• **Option 1**: Install dedicated rooftop fall arrest anchors meeting Cal/OSHA requirements into the side of the concrete pedestal at a minimum of six locations. Install a Cal/OSHA-compliant guardrail system around the existing roof hatch. Because the pedestal is composed of concrete with trace amounts of asbestos, the anchors will need to be installed using a drill with a built-in vacuum or using wet drilling methods. Fall arrest anchors would be expected to last at least 40 years.

Rooftop fall anchors are an inexpensive option that is relatively easy to install. Installing these would provide safe access for ongoing short-term maintenance work as well as for the long-term roof construction project.

Advantages	Disadvantages
<ul> <li>Less expensive than Option 1</li> <li>Relatively easy to install</li> <li>Requires less maintenance than</li> </ul>	<ul> <li>Less convenient for maintenance workers to wear harnesses and safety lanyard</li> </ul>
Option 1	

• *Option 2*: Install a guardrail system around the entire roof to match the original guardrail system configuration, but using more durable materials than the original wood framing. A metal guardrail with a high performance coating would be expected to last at least 40 years, with overcoating of the high performance coating every 20 years.

Advantages	Disadvantages
• More convenient for maintenance works to access the roof without harnesses and safety lanyards	<ul> <li>Likely requires strengthening of existing guardrail supports</li> <li>The guardrail will require periodic maintenance</li> <li>More expensive</li> </ul>





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## Wall Openings

The final design for this approach could take a variety of forms with varying impact to the appearance of the building exterior. For the purpose of presenting a basis for design for consideration, three conceptual options are presented below, however other permutations for the wall openings are possible.

#### **Option 1 - Maintain in Existing Configuration**

This option maintains all wall openings in their current configuration and adds a system to collect and discharge water that currently leaks through the openings. There is a range of fluid-applied waterproofing membrane products that could be used to construct the water collection basins described below. Many of these products require slope to prevent water from ponding on them, which can cause them to deteriorate more quickly. Other products can tolerate ponding water, but are generally more expensive. The less expensive fluid-applied waterproofing products that may deteriorate from ponding water would be expected to require overcoating every 5 to 7 years, and would likely require complete replacement within 20 years. The more durable fluid-applied waterproofing products would be expected to last 20 years, or possibly longer, with minimal maintenance. It is possible to provide slope at the water collection basins, and a cost analysis during design of this repair option would help inform the most appropriate combination of slope and waterproofing products to minimize both the cost and maintenance of this option.

This repair option includes:

- Remove corrosion and apply a protective coating to the steel frames at openings.
- Install water collection and drainage systems at each area of wall openings where leakage occurs.
  - Construct a water collection basin with a perimeter curb of sufficient height to collect and discharge water. Assume 4 to 8 inches for most openings. Sheet metal pans can be used at smaller openings located above the floor levels. Provide slope to drain where possible.
  - Construct wood and sheet metal baffles at the interior that serve to collect wind-driven rain and limit how far it can penetrate into the building interior, directing it into the collection basin.
  - Extend collection basin a sufficient distance past opening jambs, and towards the building interior to collect water that may bypass baffles. Assume 2 to 4 feet for largest openings.
  - Install a waterproofing membrane within the collection basin, and up and over the perimeter curbs. Extend the waterproofing membrane through the gap beneath the steel plate that closes the opening, including a metal flashing with a drip edge at the exterior, if possible. Consider a fluid-applied waterproofing membrane due to the complex geometry at the opening sill where the closure plates are anchored to the steel frame.
  - Depending on the extent of actual water penetration at each opening, drains and plumbing to the exterior storm drainage system on the east side of the building may also be added at the larger or more exposed locations.

Advantages	Disadvantages
<ul> <li>Likely less expensive than other more involved options</li> <li>Retains existing ventilation, without the need to evaluate the building ventilation requirements</li> </ul>	<ul> <li>May allow a moderate amount of leakage past the collection system</li> <li>Some amount of water will likely remain within the collection basins (due to lack of positive slope), evaporating into the building over time</li> <li>Waterproofing membranes will require maintenance, and may deteriorate prematurely due to the ponding water</li> </ul>



#### **Option 2 - Selective Ventilation and Closure of Openings**

This option evaluates the ventilation requirements for the building and installs louvers where needed for ventilation, and replaces the steel plates at the other openings with watertight infills. This option prioritizes reduced maintenance and construction cost. Aluminum louvers would be expected to last more than 40 years, and any sealants use to install them would need to be replaced after about 20 years. The durability of the watertight infills would depend on their specific construction, but they would generally be expected to last at least 40 years, with overcoating of the elastomeric coating every 5 to 7 years, removal and replacement of the elastomeric coating after about 20 years, and replacement of sealant after about 20 years.

This repair option includes:

- Evaluate building ventilation and hazardous materials requirements to determine appropriate treatment for each wall opening and overall ventilation of the building.
- Remove all steel plates at wall openings.
- Install aluminum ventilation louvers with animal screens where needed for ventilation.
- Infill openings that are not required for ventilation with concrete, light gauge metal framing and stucco, or steel plates without a gap at the perimeter to create watertight assemblies.
- Coordinate the locations of ventilation louvers and opening infills to optimize the ventilation requirements and to minimize potential maintenance and leakage.

Advantages	Disadvantages
• Reduces maintenance by concentrating potential leakage at louvers, while other openings will be sealed watertight	1

#### **Option 3 - Improved Weather Protection of Openings**

This option is similar to Option 2, except that louvers with sheet metal hoods, and aluminum windows are installed to match the general appearance and configuration of original openings, as shown in Figure 1, but upgrade the weather protection where feasible. Aluminum louvers would be expected to last more than 40 years, and any sealants use to install them would need to be replaced after about 20 years. Aluminum windows with a high performance coating would be expected to last more than 40 years, with replacement of the gaskets and sealant required after about 20 years. The durability of the watertight infills would depend on their construction, but they would generally be expected to last at least 40 years, with overcoating of the elastomeric coating every 5 to 7 years, removal and replacement of the elastomeric coating after about 20 years.

This repair option includes:

- Evaluate building ventilation and hazardous materials requirements to determine appropriate treatment for each wall opening.
- Coordinate locations for ventilation louvers, hoods and windows with configuration of original openings.
- Remove all steel plates at wall openings. Optionally, where hoods are to be installed but additional ventilation is not needed, the existing steel plates could remain.
- Install aluminum ventilation louvers and sheet metal hoods with animal screens similar in appearance and configuration to original louvers and hoods, adapted to meet the current ventilation requirements.
- The ventilation evaluation may determine that only some of the original louvers are required to provide sufficient ventilation.



• At equipment hoist openings that originally had removable hollow metal panels, install new watertight permanent infill panels similar in appearance to the original hollow core metal panels to fully close off access openings.

Advantages	Disadvantages
<ul> <li>Reduces maintenance by concentrating potential leakage at louvers, while other openings will be sealed watertight</li> <li>Reduced potential for leakage at louvers by protecting them with sheet metal hoods</li> </ul>	

# **BUILDING DESCRIPTION**

WJE reviewed available documents including previous construction, condition survey and repair projects at the building. Specifically, we reviewed the following documents:

- Request for Qualifications and Proposals for Processional Structural/Engineering Design Services for the Mount Umunhum radar tower Assessment, Midpeninsula Regional Open Space District, undated including Attachments A through O.
- Original construction drawings, FD Radar Facilities FPS 24/24, Indenco Engineers, Inc., July 1959.
- Civil and landscaping drawings, *Mount Umunhum Summit Project*, Restoration Design Group, July 2016.
- Architectural and structural drawings for *Mount Umunhum Interim Tower Repair Project*, Grossmann Design Group and Rutherford + Chekene Structural Engineers, July 2014.
- *Evaluation Report: Mount Umunhum Radar Tower Exterior Analysis Project*, by ZFA Structural Engineers, October 17, 2018.

The original construction drawings provide significant detail on the overall building configuration and construction, and our visual observations, discussed below in the Site Observations section, suggest that the structural and architectural features of the existing building matches them closely. The following description includes information, dimensions and other details obtained from the reviewed original construction drawings.

## Site

The building is located along a ridge connecting the east and west peaks of the mountain. The building is fully exposed in all directions and experiences very high winds, powerful storms and freezing winter temperatures. A pedestrian pathway extends from the parking area at the west of the building, to a lookout at the east mountain peak, to the southeast of the building (Figure 10). The pathway consists of compacted resin pavement and extends around the perimeter of the building with several wood benches installed adjacent to each side of the building (Figure 11). Four bollards along the edge of the sidewalk block vehicular access to the pathway from the parking area. The middle two bollards are removable to allow authorized access.



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Figure 10. Pathway (arrow) from parking area to east peak lookout adjacent to the building.



Figure 11. Wood bench installed close to exterior wall.

## Building

The radar tower is about 85-feet tall and 60-feet square in plan with three structural bays in each direction. The exterior concrete walls are 10 inches thick, with structural bays delineated by 30-inch wide pilasters that protrude 8 inches from the exterior face of the walls at the column lines and corners (four per elevation). There are four interior columns that are 36-inches square at the first floor and 32-inches square above the second floor (Figure 12). The interior columns support concrete waffle slab floors. The walls are shown to be reinforced with two curtains of reinforcing steel (Figure 13). There are five concrete floors and a sixth level, which is a steel-framed mezzanine above the fifth floor.

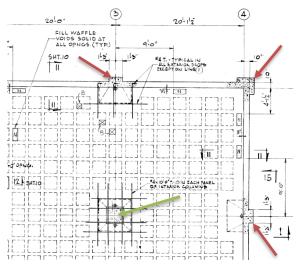


Figure 12. Partial second floor plan from original drawings showing pilasters at the sides and corner (red arrows) and interior column (green arrow).

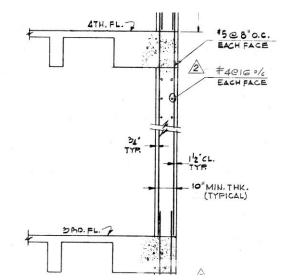


Figure 13. Partial cross section from original structural drawings showing exterior wall reinforcing.

The structural roof slab is specified to be an 8-inch thick slab with beams and an integral concrete pedestal that projects 39 inches above the top of the structural roof slab (Figure 14). The pedestal supported the radar antenna. The structural slab is topped with a built-up waterproofing membrane assembly and a 3-inch thick



concrete topping slab that is divided into four section with expansion joints. The roofing membrane and topping slab terminate at the sides of the concrete pedestal.

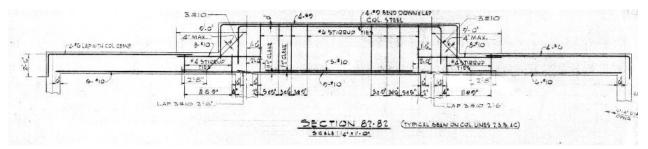


Figure 14. Cross section through roof slab showing concrete pedestal.

#### Walls

The exterior concrete walls have a variety of openings that served as windows, vents, louvers and removable panels for hoisting equipment (Figure 15). All of these openings have been closed off with steel plates or filled with concrete. The steel plates nearly fill the opening, allowing some ventilation (and moisture intrusion) into the building around their perimeter (Figure 16). Mesh installed in the perimeter gaps prevents wildlife intrusion. Reportedly some of these covers date from the building's operational period in the 1970s, but most were added in 2011 and 2016. At the top of the concrete walls, the roof slab extends past the exterior face of the walls creating a beam at the roof edge with a drip formed at the bottom edge.

At the ground level, there are three single doors at the east wall and a double door at the west, all with covered exterior entryways (wind screens) constructed of concrete block walls and reinforced concrete slab roofs (Figure 17). There are a variety of small metal items embedded in, or protruding from, the exterior of the concrete walls including abandoned pipes, brackets, inserts, bolts, and other unknown items (Figure 18), as well as a newer stacked dipole repeater antenna at the building's northeast corner.



*Figure 15. Typical opening in concrete wall, closed with steel plate.* 



Figure 16. Interior of similar opening in concrete wall with gap around steel plate perimeter.



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Figure 17. Covered exterior entryway around main entry door on west elevation. Note the spall at the edge of the roof slab.



*Figure 18. Steel fixture attached to concrete wall.* 

#### Roof

The roof of the building is dominated by a dodecagonal (12-sided) concrete pedestal (Figure 19) in the center that extends 3.25 feet above the surface of the main (lower) roof surface and directly supported the radar antenna. This pedestal is shown on the original drawings to be integral with the roof slab. A multitude of embedded or otherwise attached supports, pipes and plates associated with the former antenna protrude from the top of the pedestal (Figure 20). The main roof also has pipe penetrations around the antenna pedestal, some of which serve as a conduit through the full roof assembly, as well as eye bolts and concrete curbs at the roof edge (Figure 21 and Figure 22). There is an access hatch (Figure 23) at the southeast corner, a larger equipment hatch near the northeast corner, and a large piece of mechanical equipment (Figure 24) supported directly on the wearing slab near the south edge.



Figure 19. Concrete pedestal that once supported the radar antenna, facing northwest.



Figure 20. Steel plates, pipes, conduit, and other penetrations at the top of the concrete pedestal.



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Figure 21. Various pipe penetrations, conduit in a pitch-pocket, and visible sealant repairs at the expansion joint. Photo facing north.



*Figure 23. Roof hatch near east edge of roof, facing southeast.* 



*Figure 22. Curb with counterflashing at roof edge, facing north.* 



Figure 24. Rooftop equipment, facing southwest.

As depicted on the drawings, the perimeter of the roof originally had wood guardrails supported by steel brackets (Figure 25). The wood guardrails were possibly modified into metal guardrails (Figure 26) before being eventually removed in 2015 due to safety concerns.



*Figure 25. Perimeter brackets for supporting guardrail.* 



*Figure 26. Apparently metal guardrail that was present circa 2013.* 





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The main roofing assembly (outside of the concrete pedestal) is shown to be constructed (bottom to top) of an 8-inch-thick structural slab, a vapor barrier, 1-inch thick rigid insulation, a built-up roofing membrane, 1 inch of sand, and a 3-inch thick concrete wearing slab (Figure 27). The original built-up roofing membrane extends up the face of the concrete pedestal where it terminates behind a counterflashing set into the concrete (Figure 28 and Figure 29). The concrete pedestal does not appear to have any waterproofing provisions and was likely sheltered by the base of the radar antenna until it was removed in 1980.

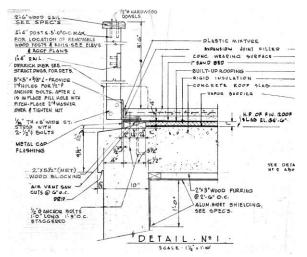


Figure 27. Roof edge detail from original drawings showing the basic configuration of the structural slab, waterproofing, topping slab, and wood guardrail.



Figure 28. Counterflashing at concrete pedestal. Note the single-ply membrane flashing that was later added, and has failed.

The roof is divided into quadrants by perpendicular expansion joints in the wearing slab (Figure 30) that do not extend through the structural slab (Figure 31). There is a roof drain (Figure 32) in the center of each quadrant, however the drain pipe was reportedly disconnected and sealed with grout to prevent animal intrusion. All four roof drains had plants growing out of them at the time of our observation.



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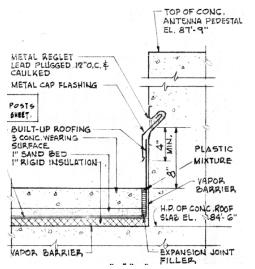


Figure 29. Roof-wall base flashing detail at concrete pedestal from original drawings.

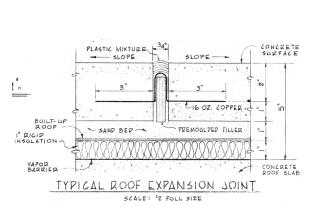


Figure 31. Roof expansion joint detail from the original drawings.



Figure 30. Wearing slab expansion joint formed from folded copper sheet that was originally covered by sealant. Note that the other three joints in the slab were concealed by more modern repair sealant.



Figure 32. One of four area drains with plants growing out of the drain.

A utility trench (Figure 33 and Figure 34) extending beneath the south end of the east wall is visible in photographs from 2013, prior to construction of the site pathway surrounding the building. This location is assumed to be where the roof drainage plumbing was reportedly disconnected and sealed with grout.



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Figure 33. Utility trenches at the southeast corner of the Radar Tower in 2013. Photo facing northwest.



Figure 34. Utility trench beneath the south end of the east elevation in 2013, prior to construction of the site pathway. Photo facing west.

#### Interior

The interior of the building has had minimal maintenance or modification since it was decommissioned in 1980. Nearly all of the furnishings and movable fixtures, and most of the other salvageable equipment and materials have been removed, but many of the partitions, ductwork, conduit, and finishes remain (Figure 35). Most of the floors are open, with some partitions remaining on the lower floors and abandoned and deteriorated HVAC equipment on some of the upper floors. There is a vertical utility shaft at the southeast corner of the building that extends the full height of the building, and a stair tower and freight elevator along the northern portion of the east wall. The fifth floor has a steel-framed mezzanine with a metal grate floor (Figure 36). There is presently no electrical power available in the building.



Figure 35. The building interior has remaining wood-framed partitions, furring and some abandoned HVAC equipment.



*Figure 36. Steel-framed mezzanine at the fifth floor.* 

## SITE OBSERVATIONS

WJE performed site investigations on March 29, April 22, and April 23, 2019, to document existing conditions at the radar tower. Our environmental consultant, SCA Environmental, accessed the building and roof on April 3, 2019, during which time samples of various materials inside the building and on the roof were taken and subsequently tested for the presence of hazardous materials. Our investigation focused



on the building enclosure, condition of the exterior surfaces of the concrete walls and roof, and limited interior areas relating to railings and accessibility for future maintenance considerations.

An unmanned aerial vehicle (drone) was used to photograph the radar tower exterior on March 29, 2019. The photos were used to develop a three-dimensional image of the radar tower and to allow for visual examination of areas that are not easily accessible by other means.

On April 22 and 23, 3019, an aerial lift was used to access the exterior concrete walls at most of the east, west and south elevations, and at limited areas at the west side of the north elevation. General conditions were noted, and the concrete was acoustically sounded with a hammer to identify concealed delaminations and voids.

The interior of the building and roof were accessed by WJE on April 22, 2019, with the assistance of District staff. The exterior walls, wall openings, roof slab, floor slabs, stairs and mezzanine were observed from the interior to document water leakage and potential hazards to maintenance personnel. The southeast portion of the roof was accessed to document conditions that require repair or will impact roofing designs.

#### **Concrete Walls**

The exterior surface of the concrete walls have an anti-graffiti coating on all four elevations up to a height of approximately 16 feet above the exterior grade. Above this level, the concrete walls are currently uncoated. The surfaces of the concrete are generally rough, weathered, and have numerous areas of poorly consolidated concrete with exposed aggregate, surface voids due to poor consolidation (honeycombing/rock pockets) (Figure 37 and Figure 38). Some areas have little or no concrete cover over reinforcing steel, with corrosion staining and spalls visible at these locations (Figure 39 and Figure 40). Also observed were debris embedded in the concrete from the original construction including pieces of wood and a cigarette lighter (Figure 41 and Figure 42). WJE sounded the surface of the concrete to identify areas of loose concrete. Areas of surface voids and loose concrete are indicated on elevation sketches for each wall, which are provided in Appendix C.



Figure 37. Poorly consolidated concrete.



Figure 38. Concrete with a rough surface and areas of poor consolidation.



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Figure 39. Corroded reinforcing steel with minimal concrete cover.



*Figure 41. Piece of wood embedded in the concrete wall on the east elevation.* 



Figure 40. Reinforcing steel with little to no cover.



Figure 42. Cigarette lighter embedded in the concrete wall on the south elevation.

Areas of poor consolidation of the concrete are typically concentrated at the base of the walls immediately above the second through fourth floor slabs (Figure 43 and Figure 44). At these locations horizontal construction joints between the slab and wall concrete placements contain a key (a small formed ridge of concrete along the top of the top of the slab along and within the thickness of the wall to enhance the mechanical bond between the separately-placed slab and wall). At numerous locations, the poorly consolidated concrete extends sufficiently deep into the wall's thickness to expose the reinforcing steel. Acoustic hammer sounding suggested a majority of the concrete adjacent to the visible areas of poorly consolidated concrete with pockets of exposed aggregate are structurally sound, with aggregate well-bonded to the wall, despite their appearance and localized small areas of loose aggregate. Some of these areas of poor consolidation appear to have been modified at some time in the past, by the addition of a trowel applied patch material, possibly following initial concrete placement. These patches were identifiable by their surface finish and coloration, which distinctly differ from the cast-in-place concrete wall (Figure 43). Some of these appear to be delaminating from the concrete substrate.



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Figure 43. Poorly consolidated concrete above the horizontal construction joint above the floor slab with a smooth patch area above.

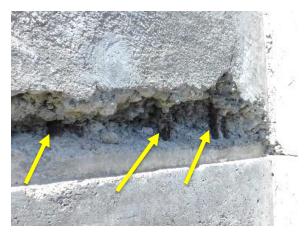


Figure 44. Poorly consolidated concrete above the horizontal construction joint above the floor slab revealing the joint key and reinforcing steel (arrows).

Diagonal cracks were observed at multiple locations on the exterior walls. At the first floor on the east side, previously patched cracks were observed between window and door openings (Figure 45) that likely formed during the Loma Prieta Earthquake in 1989, whose epicenter was only 5 miles to the southeast. The most significant of these cracks were present until about 2014 when they were repaired and the adjacent window and door openings were infilled with concrete (Figure 46).



Figure 45. Cracks visible at east elevation (arrows). (Photo taken in 2013 by WJE)

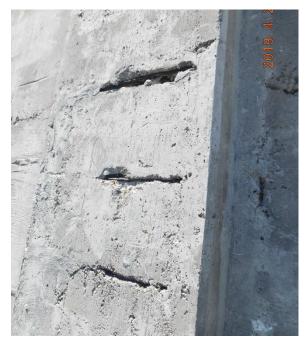


*Figure 46. Repaired cracks and infilled openings at east elevation.* 

The concrete pilasters have experienced significantly more frequent and severe concrete spalling than the surface of the walls. This spalling has exposed corroded reinforcing steel. Most of the spalling at the horizontal reinforcing is the result of inadequate concrete cover over the horizontal reinforcing (Figure 47 and Figure 48).



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*Figure 47. Concrete spalling on face of pilaster due to inadequate concrete cover.* 



*Figure 48. Concrete spalling on side of pilaster due to inadequate concrete cover.* 

In other locations where the vertical reinforcing is exposed, spalling occurs in areas of poor concrete consolidation on the sides and corners of the pilasters between the vertical reinforcing bars and the finished exterior concrete surface (Figure 49 and Figure 50).



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Figure 49. Poorly consolidated concrete on the side and corner of the pilaster. Note exposure of reinforcement steel (arrow).



Figure 50. Poorly consolidated concrete on the sides and corner of the pilaster. Note exposure of reinforcement steel (arrow).

Significant concrete spalling has occurred along the perimeter beam along the edge of the roof slab, which it protrudes from the exterior face of the walls, exposing the wall and roof slab reinforcing steel in places (Figure 51 and Figure 52). Smaller concrete spalls that are not associated with corroded reinforcing are also present, primarily along the bottom of the perimeter roof edge beam at the drip (Figure 53 and Figure 54).



Figure 51. Concrete spalling and exposed reinforcing along the perimeter roof edge beam.



Figure 52. Concrete spalling and exposed reinforcing along the perimeter roof edge beam.



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Figure 53. Small concrete spalls along the bottom edge of perimeter roof edge beam.



Figure 54. Small concrete spall along the bottom edge of perimeter roof edge beam.

Several of the concrete block wind screens were also observed to have some spalling (Figure 55). Spalling of the concrete slab forming the roof of the wind screens was also observed (Figure 56).



*Figure 55. Spalling of concrete block wall of wind screen* 

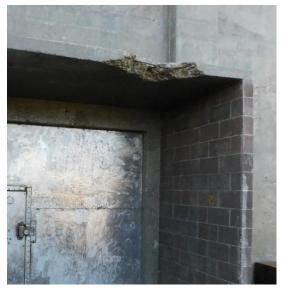


Figure 56. Spalling of concrete slab at top of roof of wind screen.

# Wall Interiors

There is widespread evidence of leakage through the concrete walls in the form of efflorescence stains at the interior of the walls and on the floor where puddles appear to routinely form based on staining patterns (Figure 57 and Figure 58). Efflorescence stains are common along the base of the interior of the concrete walls immediately surrounding the construction joint between the floor slab and wall above. These areas also tend to correlate to the pattern of poorly consolidated concrete at the exterior, which is particularly susceptible to water absorption and intrusion. Efflorescence stains were also noted in areas at the underside of floor slabs where puddles that form on top of the floor slab leak down through the slab (Figure 59). Areas of peeling paint were observed on the interior face of the exterior walls (Figure 60). At several locations



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reinforcing steel is exposed and observed to be corroded along the base of the wall (Figure 61 and Figure 62),



*Figure 57. Efflorescence stains at interior of concrete walls.* 



Figure 59. Efflorescence visible on the underside of the third floor slab where water leaks into the building and through the slab.



Figure 61. Exposed reinforcing steel observed along the base of the wall (arrows).



*Figure 58. Efflorescence stains in utility shaft at southeast building corner, facing east.* 



Figure 60. Peeling paint on the inside face of the exterior wall.



Figure 62. Exposed reinforcing steel observed along the base of the wall (arrows).



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# Wall Openings

The galvanized steel plates that cover the wall openings are generally in good to fair condition (Figure 63) however some of the original steel frames embedded into the concrete wall around openings that they are attached to are corroded, with corrosion staining below and adjacent to some openings at the interior (Figure 64). The ventilation gap around the perimeter of these plates allows water to enter the building with no means to collect and discharge it. There was standing water on the floor in several locations below these openings at the time of our visit.



*Figure 63. Galvanized steel plate with light oxidation and corroded steel frame.* 



*Figure 64. Water and corrosion staining below wall penetration.* 

Spalling of the concrete was observed on the exterior adjacent to several of the openings. At some locations spalls expose corroded reinforcing steel (Figure 65), but at most locations there is poorly consolidated concrete adjacent to the steel frames (Figure 66).



Figure 65. Concrete spalling and exposed reinforcing steel (arrow) adjacent to infilled window opening.



*Figure 66. Voids in concrete due to poor consolidation adjacent to window opening.* 

# Roof

Overall, the roof is in poor condition with multiple areas of damaged concrete and widespread surface corrosion of exposed steel embedments, penetrations and anchorages. The concrete at the pedestal is heavily





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weathered with areas of exposed aggregate and reinforcing steel, spalls and what appears to be delamination of previous concrete repairs.

The built-up roofing membrane is concealed beneath the wearing slab and could not be observed. Singleply membrane flashings have been added around the concrete pedestal and roof hatches (Figure 67), and a fluid-applied waterproofing coating has been applied to the wearing slab over the entire main roof area (Figure 68) and onto these single-ply flashings (Figure 69). There is also evidence of waterproofing repairs at underlying cracks in the concrete wearing slab, and spray foam in many joints and gaps at penetrations and flashings. There are grasses and moss growing along many joints, cracks and gaps at the roof, including plants growing out of all four roof drains (Figure 32).



Figure 67. Single-ply membrane flashing (black), spray foam (yellow), and fluid-applied coating (white) at roof equipment hatch.



Figure 68. White traffic coating on main roof, facing east. Note the darker stripes where underlying cracks were treated.



Figure 69. Damaged counterflashing at concrete pedestal showing single-ply membrane flashing (black sheet, damaged), fluid-applied coating (white), and original built-up flashing (black residue adhered to interior surface of single-ply flashing, arrow).

The concrete pedestal was likely protected by the radar antenna, relying on the mass of the concrete as the waterproofing. It does not appear to have a membrane integrated into the assembly. Large areas on the top surface and on the edges have extensive concrete spalling (Figure 70 and Figure 71).



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Figure 70. Concrete spalling and corroded steel plates on the top surface of the concrete pedestal.



Figure 71. Concrete spalling along the edge of the concrete pedestal.

# Interior

Roof drainage plumbing visible beneath the roof slab (Figure 72) runs from the four original roof drains to the utility shaft at the southeast corner of the building where it descends to the ground floor (Figure 73). The plumbing stack originally exited the building in the utility trench that extends beneath the south end of the east wall, but the pipe was reportedly disconnected and sealed with grout. The drainage plumbing has surface corrosion in many areas.



Figure 72. Cast iron drainage plumbing with widespread corrosion running into the utility shaft at the southeast corner (arrow). Photo facing south.



*Figure 73. Cast iron drainage plumbing stack in utility shaft. Photo facing southeast.* 

The ceiling beneath the roof is finished with aluminum sheets originally used as shielding (Figure 74), limiting direct observation of the underside of the structural slab. These aluminum sheets are generally oxidized, indicating long-term exposure to moisture, and there are signs of leakage at multiple areas throughout the ceiling including staining, heavier corrosion and efflorescence at the aluminum sheeting and adjacent materials (Figure 75). Several steel beams which serve to anchor steel rods extending through the roof slab that are visible at the underside of the roof have surface corrosion (Figure 76). Although the aluminum sheets prevent direct observation of the roof slab, the presence of staining of the aluminum sheets



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indicates water leakage through the roof slab and some degree of corrosion of the reinforcing steel within the roof slab is likely.

The steel-framed stair structure has surface corrosion in limited areas and many of the railings are deteriorated, corroded, or missing (Figure 77 and Figure 78). Some stair railings have been supplemented with wood railings attached with tie wire and are not of sufficient strength to reasonably perform as a guardrail (Figure 79).



Figure 74. Aluminum sheets installed beneath the roof slab are noted as radar shielding in the construction drawings.



*Figure 76. Surface corrosion on steel beam clamp.* 



Figure 75. Efflorescence deposits around penetration in concrete roof slab, with aluminum sheets in foreground.



*Figure 77. Surface corrosion on metal stair handrail.* 



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Figure 78. Missing guardrail.



Figure 79. Partially missing handrail with temporary wood railing.

The guardrails at the mezzanine above the fifth floor also show signs of corrosion (Figure 80). The soffit of the fifth floor slab exhibits staining and discoloration along exposed portions of the reinforcing steel (Figure 81).



Figure 80. Corrosion of guard rails at the mezzanine.

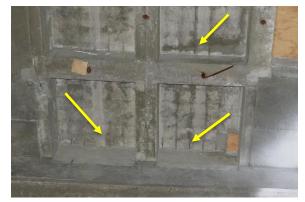


Figure 81. Corrosion staining of reinforcing steel at the soffit of the fifth floor (arrows)

Several abandoned openings (former mechanical penetrations) in the floor slabs are covered with metal plates which may date to the building's decommissioning, but most floor openings are covered with plywood fastened to the floor, reportedly installed in 2016. The metal plates typically have surface corrosion (Figure 82), and the wood covers are generally in fair condition (Figure 83), though ongoing deterioration of these wood covers should be expected given the indications of moisture intrusion throughout the building. The metal grate floor of the fifth-floor mezzanine has a number of rectangular holes (Figure 84) that someone could be injured by stepping into, and generally has surface corrosion (Figure 85).



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*Figure 82. Corroded metal plate covering opening in floor.* 



Figure 84. Rectangular holes in the mezzanine floor grating are large enough to accidentally step through.



Figure 83. Plywood covering openings in floor.



*Figure 85. Surface corrosion at mezzanine floor grating below concrete pedestal at roof above.* 

# **Exterior Doors**

Originally, there were three exterior single-leaf metal outswing doors in the east wall of the building, and a larger double-leaf outswing door opening in the center of the west wall. The two northern doors in the east wall have been removed and the openings filled with concrete. The southern door at the east wall remains (Figure 86). The door at the west is shown as a double-leaf outswing door in the original construction drawings, but currently consists of fixed metal plates that close the opening, with a single access door at the northern side (Figure 87). We are not aware of the date for this modification.

The paint has been removed from the two remaining exterior doors, exposing bare steel. Based on the absence of surface corrosion, the doors are assumed to be constructed of stainless steel. Both doors appear to be relatively secure and resistant to tampering, and have key-operated locks.



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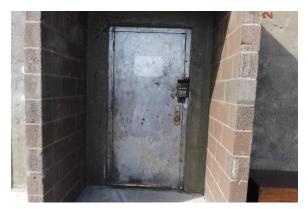


Figure 86. Steel door at south of east wall.



Figure 87. The west door was originally a double-leaf door, but appears to have been modified and has only a small access door in a larger metal plate.

# **Hazardous Materials**

On April 3, 2019, WJE's hazardous materials consultant, SCA Environmental (SCA), performed a survey of materials within the building, as well as on the building exterior, to assess the presence of hazardous materials, including asbestos, lead, and PCBs. A copy of their report is attached as Appendix A. Table 1 summarizes the locations where their testing identified detectable levels of asbestos and lead. A fire door at the elevator mechanical room is also suspected to contain asbestos within the core of the door. The specific locations of the materials are provided in the SCA report.

Table 1. Hazar dous wrater lais									
Type of Material	Location	Hazard							
Electrical wiring with black canvas	First floor interior	Asbestos							
sheathing									
Black tar at circuit breaker support	First floor interior	Asbestos							
Tan paint on concrete masonry	Second floor interior	Asbestos							
Off-white dust on the floors	Fourth and fifth floor interior	Asbestos							
Green roof coating over black roofing tar	Roof	Asbestos							
Black roofing tar under roof flashing	Roof	Asbestos							
Grey caulking	Roof	Asbestos							
Concrete pedestal for radar antenna	Roof	Asbestos							
Grey mortar for concrete masonry units	First floor exterior	Asbestos							
Off-white paint on walls and equipment	All floors	Lead							
Green paint on walls	All floors	Lead							
Yellow paint on handrails	Stairs	Lead							
Gray paint on concrete slab	First floor exterior	Lead							

#### **Table 1. Hazardous Materials**

Work activities within the building, on the roof, and at the building exterior will require special handling when the materials identified as hazardous are to be affected in addition to the District's current requirements for personal protective equipment.

## Site

The pathway from the parking area to the building has a row of four steel pipe bollards along the sidewalk to the west of the building to restrict vehicle access. The two central bollards are set into steel sleeves in



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the concrete sidewalk and can be removed (Figure 88 and Figure 89). Removing these bollards can be difficult due to sand and debris that collects between the bollard and the sleeve.

The wood benches adjacent to the building are constructed of a single large timber that is in close proximity to the building wall and the ground (Figure 90). Most of the benches are located with a gap of about 1 inch between the bench and the exterior wall of the tower. The timber benches are supported by wood bases that are reportedly well anchored to a concrete foundation below, and may be difficult or impossible to remove without damaging the benches.



*Figure 88. Bollards along the sidewalk. Arrows indicate the two removable bollards.* 



Figure 89. Base of removable bollard. Note sand in the recess adjacent to the bollard sleeve (arrow).



*Figure 90. Wood benches are very close to the concrete walls.* 



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# APPENDIX A: HAZARDOUS MATERIALS SURVEY BY SCA ENVIRONMENTAL



April 18, 2019

Mr. Brian Kehoe, SE Wiss, Janney, Elstner Associated, Inc. (WJE) 2000 Powell Street, Suite 1650 Emeryville, CA 94608

bkehoe@wje.com

Re: Non-destructive Pre-renovation Hazardous Materials Survey Mount Umunhum Radar Tower, 17000 Mt. Umunhum Road Los Gatos, CA 95030 SCA Project No.: B-12936

Dear Mr. Kehoe:

As requested, SCA Environmental, Inc. (SCA) completed a non-destructive pre-renovation survey at the abovereferenced site in Los Gatos, CA on April 3, 2019 as part of the future planned renovations. A picture of the building is shown below:



Sampling was limited to materials expected to be impacted by the renovations. Sampling was conducted by Mr. Dan Leung, CIH, CSP, a Cal/OSHA Certified Asbestos Consultant (CAC #07-4175) and a California Department of Public Health Certified Lead Inspector/Assessor (CDPH #7329). Reservoirs Environmental, Inc. (REI), an NVLAP-accredited laboratory in Denver, CO, completed bulk asbestos and lead analyses. McCampbell Analytical (McCampbell), a ELAP-accredited laboratory in Pittsburg, CA completed the bulk polychlorinated biphenyl (PCB) analysis.

Prior to any renovations or demolition, the National Emission Standard for Hazardous Air Pollutants (NESHAP) mandated by the Environmental Protection Agency (EPA) and locally enforced by the Bay Area Air Quality Management District (BAAQMD) require that all buildings be inspected for asbestos-containing materials (ACM) and materials subject to damage or which will be made friable, be removed.

#### Methodology

Asbestos sampling was performed in a fashion designed to minimize exposure of the surveyor or others to airborne asbestos fibers. Samples were typically removed from the substrate utilizing a knife or hollow drill bit bored through a wet sponge; the sample material was then placed into an airtight plastic vial. The vial's exterior was decontaminated with a wet sponge, and a unique sample I.D. written on the vial. The vial was then stored in a plastic bag. Sample substrates were patched with a high-temperature caulking compound, where required.

Samples of suspect materials were collected using triplicate sampling procedures, where applicable. Under these procedures, the first sample is analyzed. If it tests positive for asbestos (>1%), the analysis is suspended for further samples of that material. If the first sample tests only trace positive (between 0.1 to 1%), or negative, then the second and third samples are analyzed sequentially, in order to determine the possible presence of asbestos, as applicable. If all three samples test negative, the material is considered as non-asbestos. If one or more samples test "trace" positive (<1%), the material is considered to be trace positive. If one or more samples are positive for asbestos, the material is considered positive.

All asbestos samples collected were submitted to REI for analysis by polarized light microscopy with dispersion staining (DS/PLM). The Bay Area Air Quality Management District's (BAAQMD), the Federal Environmental Protection Agency's (EPA), and California Environmental Protection Agency's (Cal/EPA) regulations all specify the DS/PLM method.

#### Asbestos Standards

ACM is defined by EPA regulations as those substances containing greater than 1% asbestos. The BAAQMD and the Cal/EPA provide local enforcement of these regulations. Friable ACM with greater than 1% asbestos needs to be disposed of as asbestos waste.

Prior to demolition of a building, the BAAQMD requires abatement of friable ACM, as well as non-friable ACM that may become friable during demolition (practically, this means all non-friable ACM).

Federal Occupational Safety and Health Administrations (OSHA) regulations, locally enforced by CAL/OSHA, defines ACM as substances that contain greater than 1% asbestos. Cal/OSHA also mandates special training, medical exams, personal protective equipment and record keeping for employees working with ACM. If a material contains less than 1% asbestos but more than 0.1% asbestos, the material may be disposed of as non-ACM, but the Cal/OSHA requirements would still have to be followed regarding workers' protection and Contractor licensing.

"Trace" materials are currently regulated in California and require the following:

- Removal using wet methods;
- Prohibition of removal using abrasive saws or methods which would aerosolize the material;
- Prompt clean-up of the impacted zone, using HEPA-filtered vacuums, as applicable;
- Employer registration by Cal/OSHA for removal quantities exceeding 100 sq. ft. per year; and
- Cal/OSHA Carcinogen Registration by the Demolition or Abatement Contractor impacting such materials.

#### Lead Standards

Since elemental lead is a suspect carcinogen and known teratogen and neurotoxic in high doses, lead-containing materials need to be identified prior to the on-set of demolition activities. Using combinations of engineering controls and personal protective equipment, lead-containing materials can be remediated safely. Several sources of applicable standards are listed as follows:

1. Lead exposures in the workplace are regulated by Cal/OSHA, which has certain regulatory requirements for identifying and controlling potential lead exposures. Currently applicable regulations for the construction industry have been adopted by Cal/OSHA (8 CCR 1532.1) from the Federal OSHA

regulations. The current OSHA 8-hour Permissible Exposure Level (PEL) for lead is 50 µg/m<sup>3</sup>.

2. Current EPA and Cal/EPA regulations do <u>not</u> require LBP to be removed prior to demolition, unless loose and peeling. Provided that the paints are securely adhered to the substrates (i.e., non-flaking or non-peeling), disposal of intact demolition debris can generally be handled in California as non-hazardous and non-RCRA waste.

The applicable standards for lead are tabulated below:

Agent	Total Threshold Level Concentration (TTLC) Wet-Weight Standard (mg/kg) <sup>1</sup>	Soluble Threshold Level Concentration (STLC) Standard (mg/l) <sup>1</sup>	CalOSHA Standard for Occupational Safety
Lead	1000	5	Any detectable levels; spot abatement required from coated metals before torching/welding

In California, loose and peeling LCP or other wastes require characterization and testing for leachability. Disposal requirements are outlined as follows:

Lead Disposal Standards												
	Cl	assification a	nd Disposal	of Inorganic	Lead Wastes in	California						
Standards	TTLC	Leacha	ble Lead									
Concentations	1000 mg/kg	5 n	ng/L									
	Test M	lethods & Re	esults		Classifications							
	Total Pb	STLC Pb	TCLP Pb	Non-haz	CalHaz	Fed Haz	Stabilization	Landfill				
Condition	(mg/kg)	(mg/L)	(mg/L)	waste	(Non-RCRA)	(RCRA)	Required	Class				
1a	<50 (a1)	NA		Yes	no	no	no	III				
1b	<100 (a2)		NA	Yes	no	no	no	III				
2a		<5	<5	Yes (c)	no	no	no	III or II (d)				
2b	50 to <1000	>5	<5	no	Yes	no	no	Ι				
2c	1	>5	>5	no	Yes	Yes	Yes	Ι				
2d (b)		<5	>5	no	no	Yes	Yes	Ι				
3a		<5	<5	No	Yes	No	no	Ι				
3b	>1000	>5	<5	no	Yes	no	no	Ι				
3c		>5	>5	no	Yes	Yes	Yes	Ι				
3d (b)		<5	>5	no	no	Yes	Yes	Ι				
4	any	any	>5	no	no	Yes	Yes	Ι				

#### Lead Disposal Standards

(a1) 50 = 10 x 5 (STLC for Pb). Per WET method, impossible to exceed STLC even if 100% soluble.

(a2)  $100 = 20 \times 5$  (TCLP for Pb). Per TCLP method, impossible to exceed STLC even if 100% soluble.

(b) Physically impossible due to the stronger acid used in WET than TCLP.

(c) Landfills will likely require documentation that TCLP is <5, even though TCLP is almost always less than WET.

(d) Landfill dependent, function of permit, landfill liner, or landfill policy

- 3. The major definitions of LCP or lead-coated surfaces are listed as follows:
- a. California Department of Public Health (CDPH) defines LBP as paint that contains either  $\ge 0.5\%$  by weight of lead, or  $\ge 1$  mg/cm<sup>2</sup>.
- b. Consumer Product Safety Commission (CPSC) prohibits the manufacturing of paint that contains more than 90 ppm of lead.

Note that adherence to CalOSHA's Construction Lead Standard is required for all paint with <u>any</u> <u>measurable</u> lead content.

- 4. Lead is on the "Proposition 65" list, given its toxic potential in causing reproductive hazards.
- 5. California Department of Public Health (CDPH) requires the use of Certified Lead Workers and Supervisors for lead abatement projects at public buildings with a greater than 20 years expected life or whenever work is completed specifically to abate Lead-Based Paint. The CDPH certification requirements do not apply to this facility; however, dust controls and personnel protection are still required under 17 CCR Sections 35001 through 36100.

#### Results

Asbestos analyses by polarized light microscopy (PLM) analytical methods found the following results:

<u>Asbestos-Containing</u>: Nine (9) suspect materials were confirmed to contain asbestos that may be impacted by the renovation activities, and are listed below:

Material ID	Asbestos Materials Description
EL-1	Electrical wiring with black canvas sheathing (thin gauge)
EL-5	Black circuit breaker supports (-) w/black tar (+)
	16"x8" Off-white painted (-) w/tan compound (+) on concrete masonry unit (CMU)
WL-10	wall (-) w/gray mortar (tr)
	Dust/debris on floors (Multi-colored paint (-), off-white compound (+) and gray
DUST-13	concrete (-))
RF-15	Beige (-)/yellow (-)/ green (+)/gray (-) roof coating over black roofing tars (+)
RFMAS-17	Black roofing tars/mastic under roof flashing
CAULK-18	Gray caulking (-) along expansion joints w/black tars/mastic (+)
WL-21	16"x8" Red concrete masonry unit (CMU) wall (-) w/gray mortar (+)
CONC-19	Gray concrete (tr) radar pedestal

Note: (+) = Asbestos; (tr) = Trace Asbestos (<1%); (-) = Non-asbestos

<u>Assumed Asbestos-Containing:</u> One (1) suspect material was assumed to contain asbestos that may be impacted by the renovation activities, and will require further destructive testing. These materials are listed below:

Material ID	Asbestos Materials Description
FIREDOOR-	
AAA1	Fire-rated core in firedoor

<u>Non-Asbestos:</u> Several suspect materials that may be impacted by the renovation activities were tested or visually determined negative for asbestos, as listed below:

Material ID	Non-asbestos Materials Description
EL-2-1,2	Electrical wiring with black canvas sheathing (medium gauge)
EL-3-1	Electrical wiring with off-white canvas sheathing (thin gauge)
EL-4-1,2	Light gray/black paper behind circuit breakers
EL-6-1	Pink circuit breaker supports
EL-7-1	Black circuit breaker supports
PAINT-8-1,2,3	Off-white paint on concrete walls and equipment
GASKET-9-1	Black rubber gasket at access port of water tank
FLEX-11-1,2	Black flex connector between duct connections
CAULK-12-1,2,3	Residual beige interior caulking around window openings
VAPBAR-14-1,2	Black felts and tar vapor barrier under gray mortar bed
RF-16-1,2	Beige canvas under roof reglet
STAIR-20-1,2,3	Gray concrete steps and landing
CONC-22-1,2,3	Gray concrete perimeter walls
PAINT-23-1,2,3	Beige elastomeric exterior paint on walls in divots, cracks and behind benches

<u>Lead:</u> Paint samples collected from the interior and exterior were found to contain lead ranging from 254 to 203,819 ppm. Dust control procedures are required during demolition of painted elements to comply with the CalOSHA regulations under 8 CCR 1532.1. Torching and welding on coated items require prior spot-abatement, as required by CalOSHA.

<u>Polychlorinated Biphenyls (PCB)</u>: The levels of PCB in various caulking were found to be below or at detection (below the TTLC of 50 ppm) in the following suspect materials tested:

Material ID	Material Description of Suspect PCB Tested
CAULK-12	Residual beige interior caulking around window openings
CAULK-18	Gray caulking along expansion joints

Please contact me if you have any questions.

Sincerely, SCA ENVIRONMENTAL, INC.

Dan Leung, CIH, CSP, CAC, CDPH Vice President (415) 867-9544 dleung@sca-enviro.com

Table 1.Materials Matrix Report

Figures 1 - 2. Sample Location Diagrams

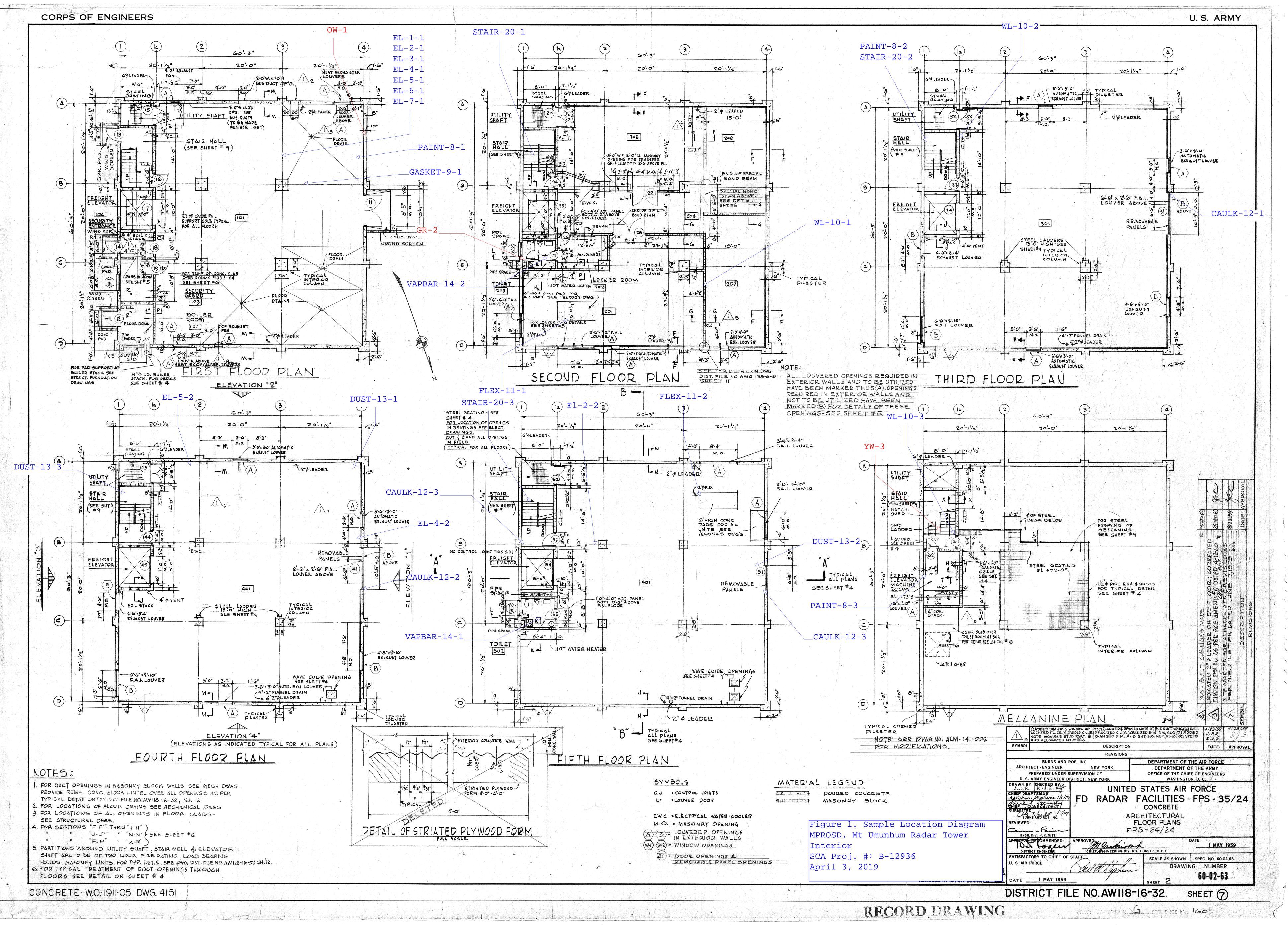
Attachments:

- 1. Asbestos Laboratory Report
- 2. Lead Laboratory Report
- 3. PCB Laboratory Report

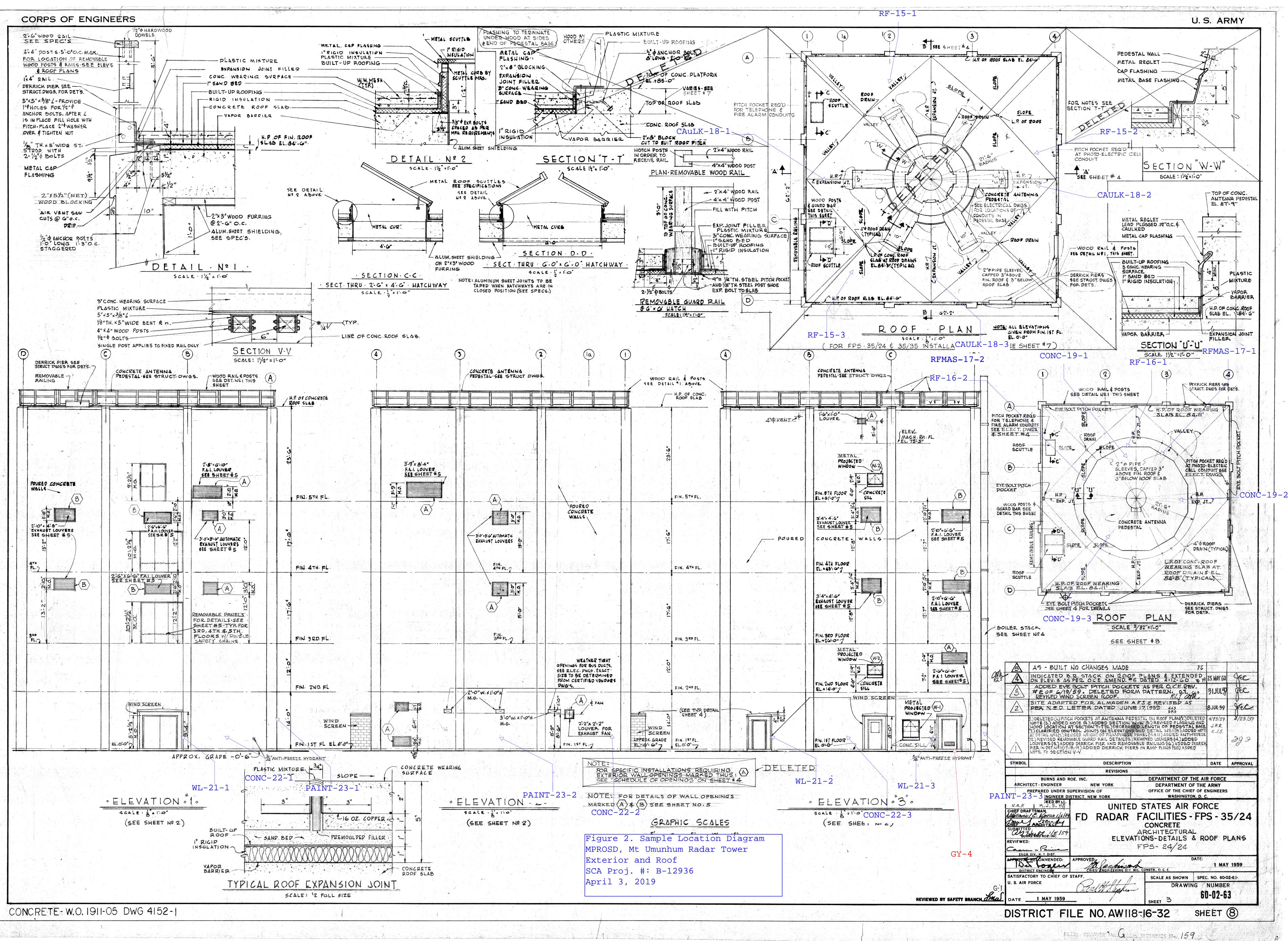
	Materials Matrix Report-MPROSD, Mt. Umunhum							_								
kadar 10	ower, 17000 Mt. Umunhum Road, Los Gatos, CA	Sub-sample #					First Floor	Second Floor	Third Floor	Fourth floor	Fifth Floor	Mezzanine	Stairs	Roof	Exterio	r
Material ID	Material Description	А	В	С	Asbestos? Positive. Trace. Assumed. Negative	UNITS (LF, SF, EA)	Inteiror	Interior	Interior	Interior	Interior	Interior	Stairs	Roof	Exterior	FOTAL (+/-15%)
													01			
ASBESTOS																
EL-1	Electrical wiring with black canvas sheathing (thin gauge)	45% CH				LF	100									100
EL-5	Black circuit breaker supports (-) w/black tar (+)	ND	40% CH			SF	100			50						150
	16"x8" Off-white painted (-) w/tan compound (+) on concrete masonry unit (CMU) wall (-) w/gray															
WL-10	mortar (tr)	4% CH in cpd	0.58% CH in mortar			SF		1200	700							1900
DUST-13	Dust/debris on floors (Multi-colored paint (-), off-white compound (+) and gray concrete (-))	6% CH	NA	NA	Positive	SF	500	500	500	500	500		800			3300
		5% CH in coating;			1 Oshtive											
RF-15	Beige (-)/yellow (-)/ green (+)/gray (-) roof coating over black roofing tars (+)	5% in tar	NA	NA		SF								4900		4900
RFMAS-17	Black roofing tars/mastic under roof flashing	35% CH	NA			SF								160		160
CAULK-18	Gray caulking (-) along expansion joints w/black tars/mastic (+)	35% CH	NA	NA		LF								150		150
WL-21	16"x8" Red concrete masonry unit (CMU) wall (-) w/gray mortar (+)	0.83% CH	ND	1.5% CH		SF									400	400
CONC-19	Gray concrete (tr) radar pedestal	ND	ND	0.58% CH	Trace	SF								750		750
	SBESTOS (Destructive Testing Required to Confirm) AA Fire-rated core in firedoor				Assumed	EA						1				1
NON-ASBEST	ГОS															
EL-2	Electrical wiring with black canvas sheathing (medium gauge)	ND	ND			LF	100									100
EL-3	Electrical wiring with off-white canvas sheathing (thin gauge)	ND			-	LF	100									100
EL-4	Light gray/black paper behind circuit breakers	ND	ND		-	SF	100			20						120
EL-6	Pink circuit breaker supports	ND				SF	100									100
EL-7	Black circuit breaker supports	ND				SF	50									50
PAINT-8	Off-white paint on concrete walls and equipment	ND	ND	ND		SF	2500	1800	1800	1800	1800	1800	320			11820
GASKET-9	Black rubber gasket at access port of water tank	ND			Negative	EA	1									1
FLEX-11	Black flex connector between duct connections	ND	ND		Inegative	EA		2			6					8
CAULK-12	Residual beige interior caulking around window openings	ND	ND	ND		LF	300	300	300	300	300					1500
VAPBAR-14	Black felts and tar vapor barrier under gray mortar bed	ND	ND			SF		100			100					200
RF-16	Beige canvas under roof reglet	ND	ND			LF								160		160
STAIR-20	Gray concrete steps and landing	ND	ND	ND		SF							800			800
CONC-22	Gray concrete perimeter walls	ND	ND	ND		SF							12800			12800
PAINT-23	Beige elastomeric exterior paint on walls in divots, cracks and behind benches	ND	ND	ND		SF							2560			2560
PCBs					PPM											
	Residual beige interior caulking around window openings				1.3	LF	300	300	300	300	300					1500
CAULK-12					< 0.50	LF								150		150
CAULK-12 CAULK-18	Gray caulking along expansion joints															
CAULK-18					PPM											
CAULK-18	Gray caulking along expansion joints Off-white paint on walls and equipment				<b>PPM</b> 254	SF	PNQ	PNQ	PNQ	PNQ	PNQ		PNQ			PNQ
CAULK-18					-	SF SF	PNQ PNQ	PNQ PNQ	PNQ PNQ	PNQ PNQ	PNQ PNQ		PNQ PNQ			PNQ PNQ
CAULK-18 LEAD OW-1	Off-white paint on walls and equipment				254		PNQ PNQ					PNQ				~

Notes: PNQ = Present, not quantified; CH = Chrysotile; ND = Not detected; NA = Not analyzed











Asbestos Laboratory Report



April 12, 2019

Subcontract Number:NALaboratory Report:REProject # / P.O. #B1Project Description:Mt.

RES 432245-1 B12936 Mt. Umunhum Radar Tower

Dan Leung SCA Environmental, Inc. 320 Justin Drive San Fransisco CA 94112

Dear Customer,

Reservoirs Environmental, Inc. is an analytical laboratory accredited for the analysis of Industrial Hygiene and Environmental matrices by the National Voluntary Laboratory Accreditation Program (NVLAP), Lab Code 101896-0 for Transmission Electron Microscopy (TEM) and Polarized Light Microscopy (PLM) analysis and the American Industrial Hygiene Association (AIHA), Lab ID 101533 - Accreditation Certificate #480 for Phase Contrast Microscopy (PCM) analysis. This laboratory is currently proficient in both Proficiency Testing and PAT programs respectively.

Reservoirs Environmental, Inc. has analyzed the following samples for asbestos content as per your request. The analysis has been completed in general accordance with the appropriate methodology as stated in the attached analysis table. The results have been submitted to your office.

**RES 432245-1** is the job number assigned to this study. This report is considered highly confidential and the sole property of the customer. Reservoirs Environmental, Inc. will not discuss any part of this study with personnel other than those of the client. The results described in this report only apply to the samples analyzed. This report must not be used to claim endorsement of products or analytical results by NVLAP or any agency of the U.S. Government. This report shall not be reproduced except in full, without written approval from Reservoirs Environmental, Inc. Samples will be disposed of after sixty days unless longer storage is requested. If you have any questions about this report, please feel free to call 303-964-1986.

Sincerely,

Jeannapencer

Jeanne Spencer President

NVLAP Lab Code 101896-0

## TABLE: PLM BULK ANALYSIS, PERCENTAGE COMPOSITION BY VOLUME

RES Job Number:	RES 432245-1
Client:	SCA Environmental, Inc.
Client Project Number / P.O.:	B12936
Client Project Description:	Mt. Umunhum Radar Tower
Date Samples Received:	April 05, 2019
Method:	EPA 600/R-93/116 - Short Report, Bulk
Turnaround:	Standard
Date Samples Analyzed:	April 08, 2019 - April 12, 2019

ND=None Detected TR=Trace, <1% Visual Estimate Trem/Act=Tremolite/Actinolite

Client Sample		Sub	Asbestos C	Content	Non Asbestos	
Number		Part	Mineral	Visual Estimate	Fibrous	Components
	R	(%)		(%)	(%)	(%)
EL-1-1	A Green resinous wire wrap	5		ND	0	100
	B Off white fibrous wire wrap w/ dark gray resinous material	95	Chrysotile	45	25	30
EL-2-1	A White fibrous material	1		ND	100	0
	B Dark gray resinous wire wrap	49		ND	0	100
	C Off white wire wrap w/ dark gray resinous material	50		ND	80	20
EL-2-2	A Tan fibrous wire wrap w/ black resinous material	10		ND	80	20
	B Black resinous wire wrap	90		ND	0	100
EL-3-1	A Light gray/red/green fibrous wire wrap	45		ND	95	5
	B Clear resinous wire wrap	55		ND		100
EL-4-1	A Gray fibrous material w/ yellow resinous material	100		ND	95	5
EL-4-2	A Black fibrous material	100		ND	85	15
EL-5-1	A Black fibrous material	100		ND	85	15
EL-5-2	A Black fibrous tar	100	Chrysotile	40	0	60

TEM Analysis recommended for organically bound material (i.e. floor tile) if PLM results are <1%.

NVLAP Lab Code 101896-0

## TABLE: PLM BULK ANALYSIS, PERCENTAGE COMPOSITION BY VOLUME

RES Job Number:	RES 432245-1
Client:	SCA Environmental, Inc.
Client Project Number / P.O.:	B12936
Client Project Description:	Mt. Umunhum Radar Tower
Date Samples Received:	April 05, 2019
Method:	EPA 600/R-93/116 - Short Report, Bulk
Turnaround:	Standard
Date Samples Analyzed:	April 08, 2019 - April 12, 2019

ND=None Detected TR=Trace, <1% Visual Estimate Trem/Act=Tremolite/Actinolite

Client		0.1	Asbestos Content	Non	
Sample Number	A Y Physical E Description R	Sub Part (%)	Mineral Visual Estimate (%)	Components	Components
EL-6-1	A Pink fibrous resinous material	100	ND	65	35
EL-7-1	A Black fibrous resinous material	100	ND	35	65
PAINT-8-1**	A White/multi-colored paint	35	ND	0	100
	B Gray granular material	65	ND	0	100
PAINT-8-2	A White/multi-colored paint w/ green compound	100	ND	0	100
PAINT-8-3	A White/multi-colored paint w/ green compound	100	ND	0	100
GASKET-9-1	A Black resinous material	100	ND	0	100
WL-10-1	A Green/multi-colored paint	TR	ND	0	100
	B Tan compound	TR	Chrysotile 4	0	96
	C Gray cinder block	40	ND		100
	D Gray granular material	60	ND	0	100

TEM Analysis recommended for organically bound material (i.e. floor tile) if PLM results are <1%.

NVLAP Lab Code 101896-0

## TABLE: PLM BULK ANALYSIS, PERCENTAGE COMPOSITION BY VOLUME

RES Job Number:	RES 432245-1
Client:	SCA Environmental, Inc.
Client Project Number / P.O.:	B12936
Client Project Description:	Mt. Umunhum Radar Tower
Date Samples Received:	April 05, 2019
Method:	EPA 600/R-93/116 - Short Report, Bulk
Turnaround:	Standard
Date Samples Analyzed:	April 08, 2019 - April 12, 2019

ND=None Detected TR=Trace, <1% Visual Estimate Trem/Act=Tremolite/Actinolite

Client	L		Asbestos	Content	Non	Non-
Sample		Sub			Asbestos	Fibrous
Number	Y Physical E Description	Pan	Mineral	Visual	Fibrous Components	
	R	(%)		Estimate (%)	(%)	(%)
WL-10-2	A Black/multi-colored paint	15		ND	0	100
	B Gray cinder block	25		ND	0	100
	C Tan gray plaster	60	Chrysotile	TR	0	100
			Point Count	0.58		
WL-10-3	A White/multi-colored paint	15		ND	0	100
	B Gray/red cinder block	85		ND	0	100
FLEX-11-1	A Black resinous material w/ white fibrous material & gray paint	100		ND	50	50
FLEX-11-2	A White paint w/ yellow resinous material	2		ND	0	100
	B Black resinous material w/ white fibrous material	98		ND	50	50
CAULK-12-1	A Green/gray paint	5		ND	0	100
	B Off white resinous material	95		ND	10	90
CAULK-12-2	A Off white-gray resinous material	100		ND	15	85

TEM Analysis recommended for organically bound material (i.e. floor tile) if PLM results are <1%.

NVLAP Lab Code 101896-0

## TABLE: PLM BULK ANALYSIS, PERCENTAGE COMPOSITION BY VOLUME

RES Job Number:	RES 432245-1
Client:	SCA Environmental, Inc.
Client Project Number / P.O.:	B12936
Client Project Description:	Mt. Umunhum Radar Tower
Date Samples Received:	April 05, 2019
Method:	EPA 600/R-93/116 - Short Report, Bulk
Turnaround:	Standard
Date Samples Analyzed:	April 08, 2019 - April 12, 2019

ND=None Detected TR=Trace, <1% Visual Estimate Trem/Act=Tremolite/Actinolite

Client		Cult	Asbestos	Content	Non	
Sample Number	A   Physical     Y   Description     R	Sub Part (%)	Mineral	Visual Estimate (%)	Components	Components
CAULK-12-3	A Green paint	5		ND	0	100
	B Gray-off white resinous material	95		ND	15	85
DUST-13-1**	A Off white compound	2	Chrysotile	6	0	94
	B Off white compound	2		ND	4	96
	C White multi-colored paint w/ multi-colored debris	10		ND	0	100
	D Gray granular debris	86		ND	0	100
DUST-13-2**	Not Analyzed per Client Request.					
DUST-13-3**	Not Analyzed per Client Request.					
VAPBAR-14-1	A White paint	1		ND	0	100
	B Black multi-layered tar	35		ND	0	100
	C Black multi-layered felt	64		ND	75	25

TEM Analysis recommended for organically bound material (i.e. floor tile) if PLM results are <1%.

NVLAP Lab Code 101896-0

## TABLE: PLM BULK ANALYSIS, PERCENTAGE COMPOSITION BY VOLUME

RES Job Number: Client:	RES 432245-1 SCA Environmental, Inc.
Client Project Number / P.O.:	B12936
Client Project Description:	Mt. Umunhum Radar Tower
Date Samples Received:	April 05, 2019
Method:	EPA 600/R-93/116 - Short Report, Bulk
Turnaround:	Standard
Date Samples Analyzed:	April 08, 2019 - April 12, 2019

ND=None Detected TR=Trace, <1% Visual Estimate Trem/Act=Tremolite/Actinolite

Client	L	Qub	Asbestos	Content	Non	
Sample Number	Y Physical E Description R	Sub Part (%)	Mineral	Visual Estimate (%)	Components	Components
VAPBAR-14-2	A Black tar w/ white paint	10		ND	0	100
	B Light gray granular material	15		ND	0	100
	C Black felt	35		ND	80	20
	D Gray plaster	40		ND	0	100
RF-15-1	A Tan resinous material	5		ND	0	100
	B Yellow resinous material	5		ND	0	100
	C Green resinous material	10	Chrysotile	5	0	95
	D Gray resinous material	30		ND	0	100
	E Black fibrous tar	50	Chrysotile	5	0	95
RF-15-2	Not Analyzed per Client Request.					
RF-15-3	Not Analyzed per Client Request.					
RF-16-1	A Off white tape w/ silver/gray resinous material	100		ND	90	10
RF-16-2	A Off white tape w/ silver/gray resinous material	100		ND	80	20

TEM Analysis recommended for organically bound material (i.e. floor tile) if PLM results are <1%.

NVLAP Lab Code 101896-0

## TABLE: PLM BULK ANALYSIS, PERCENTAGE COMPOSITION BY VOLUME

RES Job Number:	RES 432245-1
Client:	SCA Environmental, Inc.
Client Project Number / P.O.:	B12936
Client Project Description:	Mt. Umunhum Radar Tower
Date Samples Received:	April 05, 2019
Method:	EPA 600/R-93/116 - Short Report, Bulk
Turnaround:	Standard
Date Samples Analyzed:	April 08, 2019 - April 12, 2019

ND=None Detected TR=Trace, <1% Visual Estimate Trem/Act=Tremolite/Actinolite

Client	L		0.1	Asbestos	Content	Non	
Sample Number	A Y E R	Physical Description	Sub Part (%)	Mineral	Visual Estimate (%)	Components	Components
RFMAS-17-1	A	Gray/black fibrous tar	100	Chrysotile	35	0	65
RFMAS-17-2		Not Analyzed per Client Request.					
CAULK-18-1	A	Gray resinous material	50		ND	0	100
	В	Black fibrous tar	50	Chrysotile	35	0	65
CAULK-18-2		Not Analyzed per Client Request.					
CAULK-18-3		Not Analyzed per Client Request.					
CONC-19-1**	A	Off white/multi-colored granular resinous material	40		ND	0	100
	В	Gray granular material	60		ND	0	100
CONC-19-2**	A	Gray cementitious material	100		ND	0	100
CONC-19-3**	A	Gray cementitious material	100	Chrysotile	TR	0	100
				Point Count	0.58		

TEM Analysis recommended for organically bound material (i.e. floor tile) if PLM results are <1%.

NVLAP Lab Code 101896-0

## TABLE: PLM BULK ANALYSIS, PERCENTAGE COMPOSITION BY VOLUME

RES Job Number:	RES 432245-1
Client:	SCA Environmental, Inc.
Client Project Number / P.O.:	B12936
Client Project Description:	Mt. Umunhum Radar Tower
Date Samples Received:	April 05, 2019
Method:	EPA 600/R-93/116 - Short Report, Bulk
Turnaround:	Standard
Date Samples Analyzed:	April 08, 2019 - April 12, 2019

ND=None Detected TR=Trace, <1% Visual Estimate Trem/Act=Tremolite/Actinolite

Client		Sub	Asbestos	Content	Non Asbestos	
Sample Number	A     Y     Physical       E     Description       R		Mineral	Visual Estimate (%)	Fibrous	Components
STAIR-20-1	<ul> <li>A Dark gray plaster</li> <li>B White/gray paint/multi-colored paint w/ white resinous material</li> </ul>	15 35		ND ND	0 0	100 100
STAIR-20-2	C Gray granular material A White/gray/multi-colored paint	50 15		ND ND	0	100 100
STAIR-20-3**	<ul><li>B Gray granular material</li><li>A Gray granular material</li><li>B Gray/gray paint</li></ul>	85 40 60		ND ND ND	0 0 0	100 100 100
WL-21-1	A Brown cinder block B Gray granular material	40 60	Chrysotile	ND TR	0 0	100 100
WL-21-2	<ul><li>A Clear resinous material</li><li>B Brown cinder block</li><li>C Gray granular material</li></ul>	3 25 72	Point Count	0.83 ND ND ND	0 0 0	100 100 100

TEM Analysis recommended for organically bound material (i.e. floor tile) if PLM results are <1%.

NVLAP Lab Code 101896-0

## TABLE: PLM BULK ANALYSIS, PERCENTAGE COMPOSITION BY VOLUME

RES Job Number:	RES 432245-1
Client:	SCA Environmental, Inc.
Client Project Number / P.O.:	B12936
Client Project Description:	Mt. Umunhum Radar Tower
Date Samples Received:	April 05, 2019
Method:	EPA 600/R-93/116 - Short Report, Bulk
Turnaround:	Standard
Date Samples Analyzed:	April 08, 2019 - April 12, 2019

ND=None Detected TR=Trace, <1% Visual Estimate Trem/Act=Tremolite/Actinolite

Client	L	Cub	Asbestos (	Content	Non Asbestos	
Sample Number	Y Physical E Description R	Sub Part (%)	Mineral	Visual Estimate (%)	Fibrous	Components
WL-21-3**	A Gray granular material	25	Chrysotile	TR	0	100
	B Brown granular material	75	Point Count	1.50 ND	0	100
CONC-22-1	A Clear resinous material	5		ND	0	100
	B Gray cementitious material	95		ND	0	100
CONC-22-2	A Tan-gray cementitious material	35		ND	0	100
	B Dark gray granular material w/ clear resinous material	65		ND	0	100
CONC-22-3**	A Clear resinous material	2		ND	0	100
	B Gray granular material	98		ND	0	100
PAINT-23-1	A Clear resinous material	10		ND	0	100
	B Off white granular material	90		ND	0	100
PAINT-23-2	A Clear resinous material	25		ND	0	100
	B Off white granular material	75		ND	0	100

TEM Analysis recommended for organically bound material (i.e. floor tile) if PLM results are <1%.

NVLAP Lab Code 101896-0

## TABLE: PLM BULK ANALYSIS, PERCENTAGE COMPOSITION BY VOLUME

RES Job Number: Client: Client Project Number / P.O.: Client Project Description: Date Samples Received: Method: Turnaround: Date Samples Analyzed:	RES 432245-1 SCA Environme B12936 Mt. Umunhum F April 05, 2019 EPA 600/R-93/1 Standard April 08, 2019 -	Radar 16 - S	Tower Short Report, Bulk			TR	D=None Detected ==Trace, <1% Vi em/Act=Tremolit	sual Estimate
Client Sample Number		L A Y E R	Physical Description	Sub Part (%)	Asbestos Mineral	Content Visual Estimate (%)	Non Asbestos Fibrous Components (%)	Fibrous Components
PAINT-23-3		-	lear resinous material ff white granular material	50 50		ND ND	0 0	100 100

TEM Analysis recommended for organically bound material (i.e. floor tile) if PLM results are <1%.

Anh Jugg Anita Grigg

Analyst / Data QA

Due Date Due Time:

# Reservoirs Environmental, Inc. 5801 Logan St. Denver, CO 80216 · Ph: 303 964-1986 · Fax 303-477-4275 · Toll Free :866 RESI-ENV

Atta	ichme	nt 1		
RES 43	3 <mark>22</mark> 4	5		3
Page _	1	of	_3_	6

SUBMITTED BY:	INVOICE TO: (IF DIFFE	REN	IT)									CON	TACT IN	FOR	-	ON:				
Company: SCA Environmental, Inc.	Company: SAME	Contact: Dan Leung										Contact:								
Address: 320 Justin Drive	Address:		Phone:											Phone:						
San Fransisco CA 94112						Fax	k: Il/pager:							_	Fax: Cell/pag	or				
Project Number and/or P.O. # B12036						-	0.200.000000000	ta Doliv	erable E	mail Ad	droce				Cempa	ю.				
Project Number and/or P.O. #: B12936 Project Description/Location: Mt. Umunhum Radar Tower						-1"	nai Da	ta Deliv	erable L	Inan Au	uress	<b>1</b>								
																	00000			
ASBESTOS LABORATORY HOURS: Weekdays:						R	EQUE	STED	ANAL	YSIS				1.1.2	negative and the second	ATRIX C	Bulk = B	LAB NOTES:		
PLM / PCM / TEMRUSH (Same Day)PRIOF (Rush PCM = 2h)	RITY (Next Day) x STANDARD (3-5 Day)													Air = )ust =	11222		Paint = P			
CHEMISTRY LABORATORY HOURS: Weekdays			Dust),					= 5		ъ				Soil =		-	Wipe = W			
Metal(s) / Dust**RUSH24 h			or D					+/- or	c 0	N			Sw	ab =	SW		F = Food			
RCRA 8 / Metals & Welding RUSH (3 Day ) 5	**Prior notification is Day 10 Day required for RUSH		Bulk reps			님		ella, ter:	Quantification Yes / No	ID, Y &		tion ,	Drinkin	g Wa	ter = D	W Wast	e Water = WW			
Fume Scan / TCLP**	turnarounds.**		50			Can,		phlobac	antifies			nmental Identification B DTES:			11.11.11.11.11.11.11.11.11.11.11.11.11.	= Other				
Organics24 hr3 da		_				Is S		nphl	a ×	e Col	-	nmei ident a DTES	**AS	TME	1792 ap	proved wip	e media only**			
MICROBIOLOGY LABORATORY HOURS: Weeko E.coli and/or Coliforms* 24-48 Hour Of	lays: 9am - 6pm ther:	-	ISO, +/- (A SO-Indirect			Metals Scan,		s, Camp	or One)	c Plate Count Quantification	Jantification	or Environmental : +/- , Identifica Von-Viable THER NOTES:								
Pathogens*24-48 Hour				A		ne,		Plate C aureus	+/-	obic	Intific	or E ton-/								
Microbial Growth*5-10 Day	*TAT dependent on speed of microbial growth.*		. 74 ro-va	OSHA	ole	L.	0	S.au	e Ci	Aer /- c	ō	Bulk or h	Area							
Legionella 10 Day	microbial growth.		Vel II Mici	OB,	Respirable	(s) elding	TS:	eria,	Colifo	al, +	+/- OL	den, jable LS C	11(-							
	Hr3 Day5 Day	report	, Le	7400B,	Res	alyte(	HE .	Lister Lister	ter (F	Fung	Ť	obur on, V IITIA	ne (I							
**Turnaround times establish a laboratory priority, guaranteed. Additional fees apply for after		Short r	AHERA, Level II, 7402 Semi-Quant, Micro-vac,	7400A,	otal,	- Analyte(s) TCLP, Welding Fume,	N- N	oger 7:H7,	i anc Wa	obia eria,	onella	Other: Bioburden, old: Spore Trap or LER'S INITIALS C	olun	Code	ers					
	ayStandard	- s			- Total,	V.8. 7	ORGANICS - METH, TSS	Pathogens: Aerobic Pathogens: Aerobic Pathogens: Active Contention Content Con	E.coli and/or Coliforms: +/- or State Water (Please Circle One)	Microbial Growth: Aerobic Bacteria, Fungal, +/- or Q	Legionella:	Other: Bioburden, LAL or Environment Mold: Spore Trap or Bulk: +/-, Identifi Quantification, Viable or Non-Viable MPLER'S INITIALS OR OTHER NOTES:	Sample Volume (L) / Area	C X C	Containers	Date	Time	EM Number		
	)'s must be unique)	PLM	TEM - Quant,	PCM	DUST	METALS RCRA 8,	ORG		Via	bles ROBIOL	OGY		Sam	Matrix	ů #	Collected mm/dd/yy	Collected	(Laboratory Use Only)		
1 EL-1-1		X			-									B		,,				
		X												В						
2 EL-2-1														B						
3 EL-2-2		X	410004100000																	
4 EL-3-1		×												В						
5 EL-4-1		X												B						
6 EL-4-2		X												В						
7 EL-5-1		X												В						
8 EL-5-2		X												В						
9 EL-6-1		X												В						
10 EL-7-1		X												в						
Number of samples received: 52	(Additional samples shall be listed o	n atta	ched lo	ng fo	rm.)									- fouristic						
NOTE: REI will analyze incoming samples based upon information re	eceived and will not be responsible for errors or omissions in	n calcu	lations res	sulting	from th	he inac	curacy o	of original	data. By	signing cl	ient/con	npany representati	ve agrees tha	t subn	nission o	f the followin	ng samples for reque	ested analysis as indicated		
on this Chain of Custody shall constitute an analytical services agree	ment with payment terms of ME1 30 days, failure to comply	with ba	ayment ter	ma ma	ayrest	artinid	1.076 mil	anany nite	a car aurci	199.		<b>`</b>								

Relinquis	shed By: 🌙	Ugiled 0				Date/	Time:	1.1	$\sim$	)	Sample Condition:	On Ice	Sealed Intact
Laborato Received B	y: Use Only	Chece Lok	Date	/Time: 4/6/19	9:35-	4	Carrier:	Hand / FedEx Box	UPS /		Temp. (F°)	_ Yes / No	Yes / No Yes / No
Data Entry	Contact	Phone Email Fax	Date	Time	Initials	Contact		Phone Email Fax		Da	te	Time	Initials
QA:	Contact	Phone Email Fax	Date	Time	Initials	Contact		Phone Email Fax		Da	te	Time	Initials

				REQUESTED ANALYSIS													Attachme					
Denem wine Frankright Int	_ [														Air =			ulk = B	LAB NOTES:			
REILAB Reservoirs Environmental, Inc. 5801 Logan St. Denver, CO 80216 - Phr. 303 964 - 1986 - Fax 303 - 477 - 4275 - 101 Free :866 RESI-ENV		8					Е.co +/-	No N	Y &													
		Quant,					nella,	ificat	D,			ition,			Dust Soil :			aint = P ipe = W	2			
	ton	on			can		lobad	Quantification Yes / No	Dut	ation	ental	ntifica				= SW		= Food				
	Point Count	ISO, +/-, rect Preps	9		als S		nt, Sa amph		or Co	Quantification	onme	ble	OTES	Drinkin	ng Wa		and with the second	Water = WW				
RES #: 432245-1         Page _2 of _ 3					Met		S, Cour	e O o	C Pla	Qua	Invin	/-, I-Vial	R N				D = Other pproved wipe media only**					
	report	402, D-Indi	AH		'aur		Plate Count, Salmonella, E.coli aureus, Camphlobacter: +/-	s: +/ Circl	erobi	ĩ	LAL or Environmental	k: + r Nor	DTHE	-AS I	IME	1/92 a	pproved wipe	media only**				
Submitted by: SCA Environmental, Inc.	- Short report   ono	- AHERA, Level II, -quant, Micro-vac, IS	M - 7400A, 7400B, OSHA	DUST - Total, Respirable	METALS - Analyte(s) RCRA 8, TCLP, Welding Fume, Metals Scan	I CLP, Welding F	0 10	E.coli and/or Coliforms: +/- or State Water (Please Circle One)	aqter (Please Circle Une) Yes / No Microbial Growth: Aerobic Plate Count ID, Y & M or Bacteria, Fungal, +/- or Quantification	or bacteria, runga Legionella: +/- or	Other: Bioburden, LAI	Mold: Spore Trap or Bulk: +/-, Identification, Quantification, Viable or Non-Viable	MPLER'S INITIALS OR C	Sample Volume (L) / Area	Matrix Code	Containers	Date Collected mm/dd/yy	Time Collected	EM Number (Laboratory Use Only)			
Client sample ID number (Sample ID's must be unique)	2 ā		PCM	3	ME	S.		MICF	ROBIOL	LOGY			SA	(L) Sa		#						
11 PAINT-8-1	>					-		1000000							B							
12 PAINT-8-2	>														В							
13 PAINT-8-3	>														B							
14 GASKET-9-1	>														В							
15 WL-10-1	>														B							
16 WL-10-2	>	1.													В							
17 WL-10-3	>												****	-	B	-		5101014941010000000000000000000000000000				
18 FLEX-11-1	>								_						В							
19 FLEX-11-2	>														B							
20 CAULK-12-1	>														В							
21 CAULK-12-2	>														B							
22 CAULK-12-3	>	and the second													В							
23 DUST-13-1	>			-				untounnoi	anantaana			muummi			B							
24 DUST-13-2	>														В							
25 DUST-13-3	>														В							
26 VAPBAR-14-1	>	3													В							
27 VAPBAR-14-2	>														B							
28 RF-15-1	>														В							
29 RF-15-2	>														B							
30 RF-15-3	>	101 101 101 101 101 101 101 101 101 101													В							
31 RF-16-1	>														B							
32 RF-16-2	>														В							
33 RFMAS-17-1	×														B							
34 RFMAS-17-2	×														B							
35 CAULK-18-1	×														B							
36 CAULK-18-2	×														В							
37 CAULK-18-3	×														В							
38 CONC-19-1	×														В							
39 CONC-19-2	×														В							
40 CONC-19-3	×														В							
41 STAIR-20-1	×														В							

					REC	QUE	STED A	ANAL	YSIS						VAL	ID N	ATRIX CO	DESttachn	ent LAB NOTES:
REILAB Reservoirs Environmental. Inc.							E.coli +/-		& M						Air =			ulk = B	٢
REILAB Reservoirs Environmental, Inc. 5801 Logan St. Denver, CO 80216 • Ph: 303 964-1986 • Fax 303-477-4275 • Toll Free :866 RESI-ENV		mest			1		ці <del>і</del> і і і і	Quantification Yes /	Plate Count ID, Y & M			ć		C	)ust =	= D	P	aint = P	
-	=	Quant,			-		onella	Yes	t ID,	ç	77	Identification, le			Soil =		1000	ipe = W	
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<b>RES #: 432245-1</b> Page <u>3</u> of 3	Point Count	ct Pre			Metals		an t'	ne)	late (	Quantification	viron	ld lable	NOT	Drinkin	g Wat		DW Waste	Water = WW	
		2, IS ndirec	-		le, M		te Co eus,	+/- Ircle (	bic P	ā	or En	ulk: +/-, Id or Non-Viable	HER	**AST	M E17		oproved wipe	media only**	
	- Short report, Long report,	740 ISO-I	7400B, OSHA	e	Fur		ic Pla S.aur	Coliforms: +/- or (Please Circle One)	Microbial Growth: Aerobic or Bacteria, Fungal, +/	o	Other: Bioburden, LAL or Environmental	Bulk: le or h	R OT						
Submitted by: SCA Environmental, Inc.	Lon	vel II, /ac,	OB,	Respirable	(s)	TSS	Aerob eria, cation	Coliro (Plea	Fu	4	den,	ip or Bu Viable	LSO						
	sport	, Le icro-v		Re	alyte	H	ns: A , List	ater	al Gre		iobur	e Tra on,	ITIA	ne					
	out re	HERA It, M	7400A,	- Total,	- An TCLF	S - N	hoge 57:H7 Qui	te Wa	Bact	Legionella: +/-	ler. B	Spore Trap ification, VI	VI S.X	/olur	ode	Jers	Date	Time	
		TEM - AHERA, Level II, 7402, ISO, +/-, Semi-quant, Micro-vac, ISO-Indirect Preps		2	METALS - Analyte(s) RCRA 8, TCLP, Welding Fume,	<b>ORGANICS - METH, TSS</b>	Pathogens: Aerobic Plate Cou 0157:H7, Listeria, S.aureus, C or Quantification	Sta C	ables	Leg	đ	Mold: Spore T Quantification,	SAMPLER'S INITIALS OR OTHER NOTES	ple \	Matrix Code	# Containers	Collected mm/dd/yy	Collected hh/mm a/p	EM Number (Laboratory Use Only)
Client sample ID number (Sample ID's must be unique)	PLM	TEM	PCM	DUST	RCR	ORG		a sublicit	ROBIO	LOGY		20	SAM	Sample Volume (L) / Area	Matr	0 #	Collected &		(Edeoratory one only)
42 STAIR-20-2	X														В				
43 STAIR-20-3	X														В				
44 WL-21-1	X														В				
45 WL-21-2	X														В				
46 WL-21-3	X										*1000000				B				
47 CONC-22-1	X														B				
48 CONC-22-2	X														B				
49 CONC-22-3	X														B				
50 PAINT-23-1 51 PAINT-23-2	X X														B				
51 PAINT-23-2 52 PAINT-23-3	X														B				
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Bill to: SCA EMAIL HEADING: WSE M UMUN RADAR MUR LAB REL COURIER LAB REP NOTIFIED: AIRBILL/FLIGHT NO: EST ARRIVAL DATE: Method Reference Sample Media RESULTS DUE: CHAIN OF CUSTODY Sending Info Received by Lab:	7400 PCM PLM (asbestos	Notification DATE/TIME Shipper REFERENCE I.D. EST. ARRIVAL TIME AHERA TEM (<0.005 s/cc AnaSen) Flame AA (Lead)	Site Name/Address) - (Da MI UMUNHUM PADAR IOUER CARB-AHERA TEM 0.001 s/cc A		Dan Leung eports99@gmail.com	(PROJ MGR) (TECH) (ACCT)
RADAR AWR LAB REL LAB REP NOTIFIED: AIRBILL/FLIGHT NO.: EST ARRIVAL DATE: Method Reference Sample Media RESULTS DUE: CHAIN OF CUSTODY Sending Info	7400 PCM PLM (asbestos 25 37 mm	Notification DATE/TIME Shipper REFERENCE I.D. EST. ARRIVAL TIME AHERA TEM (<0.005 s/cc AnaSen) Flame AA (Lead)	PADAR JOWER		eports99@gmail.com	
LAB REL COURIER LAB REP NOTIFIED: AIRBILL/FLIGHT NO.: EST ARRIVAL DATE: Method Reference Sample Media RESULTS DUE: CHAIN OF CUSTODY Sending Info	7400 PCM PLM (asbestos) 25 37 mm	Notification DATE/TIME: Shipper REFERENCE I.D. EST. ARRIVAL TIME: AHERA TEM (<0.005 v/cc AnaSen) Flame AA (Lead)	PADAR 1046R		RUCTIONS TO LAB:	(ACCT)
COURIER LAB REP NOTIFIED: AIRBILL/FLIGHT NO.: EST ARRIVAL DATE: Method Reference Sample Media RESULTS DUE: CHAIN OF CUSTODY Sending Info	PLM (asbestos) 25 37 mm	Shipper REFERENCE I.D. EST. ARRIVAL TIME AHERA TEM (<0.005 %/cc AnaSen) Flame AA (Lead)	CARB-AHERA TEM 0.001 s/cc A			
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Sample Media RESULTS DUE: THAIN OF CUSTODY Sending Info	PLM (asbestos) 25 37 mm	Flame AA (Lead)	CARB-AHERA TEM 0.001 s/cc A		P1 (1 (1200 P13)	
RESULTS DUE: THAIN OF CUSTODY Sending Info	25 37 mm	9	CID ALC (L. )	a Sensitivity	W/REGULTS < 1º/0	
ESULTS DUE: HAIN OF CUSTODY Sending Info		0.45 0.8 micron	CP/MS (Lead) MCEF Bulk Water Wipe		/ 10000100 110	
CHAIN OF CUSTODY Sending Info	JENTS	AM / PM	acer (bank) water wipe	1		1
U C	DATA	AM / FM	11, 1			
and the Same street	52 sam	ples submitted by	on 4/4 at 4:0	P		
	-	ples received by	on 415 at 9!	35A		
Received by Analyst:	sam	ples received by	on at			
AMPLE ID	LITERS	Results GAMPLE ID	ns/Blanks/Outs	_		
EL-1-1		61AIR - 20 - 1, 2,3				
EL-2-1,2		WL-21-1,2,3				
EL-3-1		CONC-22-1,23		1		1
EL-4-1.2		PAINT-23-1,2,3				
EL-5-1,2						
EL-6-1						
11-1-1	-					
PAINT-8-1,2	. 3					
GAGKET-9-1						
WL-10-1,2.3	-					
FLEX -11-1,2						1
the second se	3					
DUGT-13-1,2,	-					
VAPBAR- 14 - 1.	4					
RF-15-1,2,3	-					
RF-16-112			DLANK			
RFMAG-17-1,2			BLANK	1		
CAULK- 18 - 1,2,3			BLANK	1		
		ot applicable AND circle items ap		1		1
1. Pickup requested:	- (					
Contact			Time of Call:			
2. Call contact to acknow 3. Analyze samples by P		samptes.				
4. Analyze inside sam	ples by PCM fin	rst; if any sample >0.01 f/cc, c	ontact project manager.			
		rith items 6, 7 or 8, as noted. f Avg >70 str/mm^2, contact PM	before analyzing outsides of	blanks		
7. Analyze all samples, i						
8. Do NOT analyze outs			Y	1		
		mple with the highest PCM resul e (>1%); first trace (<0.1%);exe		ples.		
II. Analyze all bulk sam						
		norized to perform Florisil cleanup and REGULATED ASBESTOS.	Soxhiet extraction to meet the det	ction amit.		
Report Number:		Supplies /Equipment	Qty			
		Hi-Vol (3040)				
		Lo-Vol (3020)				
nvoice Number:		TEM / Pb cassettes (3520)		1		1
avoice rumber:		PCM cassettes (3500)				
		Bulk sampling supply (3710)	52			

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#### Lead Laboratory Report



April 9, 2019

Laboratory Code: Subcontract Number: Laboratory Report: Project # / PO #: Project Description:

RES NA RES 432224-1 B12936 Mt. Umunhum Radar Tower

Dan Leung SCA Environmental, Inc. 320 Justin Drive San Fransisco CA 94112

Dear Customer,

Reservoirs Environmental, Inc. is an analytical laboratory accredited for the analysis of Industrial Hygiene and Environmental matrices by the American Industrial Hygiene Association, Lab ID 101533 - Accreditation Certificate #480. The laboratory is currently proficient in both IHPAT & ELPAT programs respectively.

Reservoirs has analyzed the following sample(s) using Atomic Absorption Spectroscopy (AAS) / Inductively Coupled Plasma - Mass Spectrometry (ICP-MS) per your request. Reported sample results were not blank corrected. The analysis has been completed in general accordance with the appropriate methodology as stated in the analysis table. Results have been sent to your office.

**RES 432224-1** is the job number assigned to this study. This report is considered highly confidential and the sole property of the customer. Reservoirs Environmental, Inc. will not discuss any part of this study with personnel other than those authorized by the client. The results described in this report only apply to the samples analyzed. This report shall not be reproduced except in full, without written approval from Reservoirs Environmental, Inc. Samples will be disposed of after sixty days unless longer storage is requested. If you should have any questions about this report, please feel free to call me at 303-964-1986.

Sincerely,

pence

Jeanne Spencer President

### RESERVOIRS ENVIRONMENTAL, INC. 5801 Logan St., Suite 100 Denver CO 80216

#### TABLEANALYSIS:

#### LEAD IN BULK

RES Job Number:	<b>RES 432224-1</b>
Client:	SCA Environmental, Inc.
Client Project Number / P.O.:	B12936
Client Project Description:	Mt. Umunhum Radar Tower
Date Samples Received:	April 5, 2019
Analysis Type:	REI CHEMISTRY SOP / USEPA SW846 3050B/6020A
Turnaround:	Standard
Date Samples Analyzed:	April 9, 2019

Client	Reporting	LEAD
ID Number	Limit	CONCENTRATION
	(mg/kg)	(mg/kg)
OW-1	0.59	254
GR-2	0.66	2,027
YW-3	0.65	203,819
GY-4	0.63	5,526

\* Unless otherwise noted, all quality control samples are performed within specifications established by the laboratory.

David E. Monagle

Analyst / Data QA:\_

Due Date Due Time:

Contact

Phone Email Fax

## REILAB Reservoirs Environmental, Inc.

Attachment 1 RES 432224 De

Date

			ter H	lours C						1-4215 •	Ton Free	.000 M	LOPEN		TACT IN	FOR	MAT		Fage	0	
SUBMITTE		INVOICE TO: (IF DIFFE	REN	NT)	-	-	Ice	CONTACT INFOR								Conta				11000	
Contraction of the local division of the loc	SCA Environmental, Inc.	Company: SAME Address:	-	Phone:						Phone:											
	320 Justin Drive	Aug 655.		Fax:						Fax:											
	San Fransisco CA 94112		_		-	Cell/pager:								Cell/p	ager:						
Project Number			-	-	-		F	inal D	ata Deliv	verable E	Email Ad	dress:			-						
Project Number				-	-		-										1			10.14	
	S LABORATORY HOURS: Weekdays: 7am - 7pn	& Sat. 8am - 5pm	T		-	111111	R	REQU	ESTED	ANAL	YSIS					VAL		MATRIX CO	DDES	LA	B NOTES:
PLM / PCM										12.75						Air =	A	B	Bulk = B		
FLW / FGW	(Rush PCM = 2hr, TEM = 6hr		N.		1											Dust =			aint = P		
CHEMIST	RY LABORATORY HOURS: Weekdays: 8am - 5p	n	litat	Dust),					- or		A or			100		Soil =			/ipe = W		
Metal(s) / D		iy .	ono	5					÷.	E S	& M					vab =			= Food		
	letals & Welding	**Prior notification is av required for RUSH	report,	Bulk			Ha		nella cter.		.Ö		ation		Drinkin	g Wat		O = Other	Water = WW		
Fume Scan	/ TCLP**	turnarounds.**	rep	Air,			can		loba	Quantificat Yes /			Identification	ŝ	**AS	TME		approved wipe	media only**	-	
Organics	24 hr3 day5 Day	•	Long	++-			Metals Scan.		nph mph		c Plate Count Quantification	ç	+/-, Identifica	ote		1					
	DLOGY LABORATORY HOURS: Weekdays: 9am	- 6pm	100	0 4					Count, is, Cam	- or One)	Plate	+/- or Quantification		or Non-Viable R OTHER NO	1						
			Point Count,	7402, -vac, I	¥		ICP	in the second se	Plate ( aureus	i and/or Coliforms: +/- Water (Please Circle C	Aerobic - or C	antifi	ö	Non-							
Pathogens	TAI depen	lent on speed of	int	74	OSHA	e	/te(s) Pb by Welding Fur		13	e Ci	Aer	Oue		SR C	Area	1					
Microbial G		al growth.*			8	pirat	s) Pt	TSST	erob eria,	olifo	wth al, +	-/- OL	ten, p or	lable LS C	1					1	
Legionella Mold	10 Day RUSH24 Hr48 Hr3 Day	5 Day	port	Le,	7400B.	Res	lyte(	H	s: A	or C er (P	I Growth: Fungal, +/		Tra	n, V	l) ei						
**Turna	round times establish a laboratory priority, subject to	aboratory volume and are not	1 r	RA.	A.	al.	- Analyte(s) Pb t TCI P Walding F	- W	gen H7,	and/ Wate	bial na, F	lella	er: Bioburde Spore Trap	S IN	olum	e	S			1	
	guaranteed. Additional fees apply for afterhours, wee	kends and holidays.**	Short report,	AHERA, Semi-Qui	7400A.	Total,	·F	ICS .	Pathogens: Aerobic F 0157:H7, Listeria, S.a Quantification	E.coli and/or State Water (	Microbial ( Bacteria, Fi	Legionella:	Other: old: Sp	ER	e Vo	Code	aine	Data	Time	EN	A Number
Special Ins	5 Day Stand	ard	- WL	' ਦ	PCM -	- TSUG	METALS RCRAB	ORGANICS - METH, TSS	200	Via	bles	THINK	Mold	Quantification, Viable or Non-Viable SAMPLER'S INITIALS OR OTHER NOTES:	Sample Volume (L) / Area	Matrix	# Containers	Date Collected mm/dd/yy	Collected	(Labor	ratory Use Only)
	mple ID number (Sample ID's must be u	nique)	ā	Fð	ā	ā		-	-	MIC	ROBIOI	UGT	-	S	0)	B		minudu/yy	invinit wp		
1 OW-1			Les sur		-	(clabole	X	171101151								B				Transition	CHARLEN CHI
2 GR-2							x					i suite				IN THE R					
3 YW-3							x								-	В	-				
4 GY-4							x									В					
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9				-																	
			10	<b>H</b> HHH																	
10						(	T11155				. La mani								Trainin the second	. Contractions	
	samples received: 4 (/ REI will analyze incoming samples based upon information received and will hain of Custody shall constitute an analytical services agreement with paym	Additional samples shall be listed of not be responsible for errors or omissions ent terms of NET 30 days, failure to comply	in calc	ulations re	esultin	a from	the ina	accurac a 1.5%	cy of origination of origination of origination of the second sec	al data. By terest sur	signing c	lient/cor	npany r	epresentativ	ve agrees th	at subr	missio	n of the followin	g samples for requ	ested analy	ysis as indicated
						ate/Ti						-	2		S	ample	e Cor	dition:		Sealed	Intact
	ished By: ory Use Only unu Al	Date/Time: 4/5/19 -	10	109.		11	Carr	ier:	Han	d / F∈		UPS		SPS / I	Drop T	emp.	(F°)	<u> </u>	Yes / No	es / No	Yes No
Data Entry			nitials	c	onta	ct			Phone	Email	Fax				Date			Tin		Init	NH1222
QA:	Contact	ata Timo Ir	itiale		onta	ct			Phone	Email	Fax				Date			Tin	ne	init	ials

Phone Email Fax

Contact

Initials

Time

Date

RES 432224 Attachment 1

(	CHAIN O	F CUSTODY FOR	RM		t/COC/Invoid	
II to: GCA			1 Y	DAN	LEUNG	(PROJ MGR
			10/00	Dan l	Leung	(TECH
AIL HEADING:	(Project #) -	Project Manager Initials) -	(Site Name/Address) - (Date MMDD)			
ADAR TUR SI	UM B1299	PP DL			-	ACCT
	1/		RADAR YOWER	labreports99	@gmail.com	(ACCT
B REI				INSTRUCTIONS	TO LAB:	
URIER		Notification DATE/TIME				
LAB REP NOTIFIED: AIRBILL/FLIGHT NO.:		Shipper REFERENCE LD				
EST ARRIVAL DATE:		EST. ARRIVAL TIME				
thod Reference	7400 PCM		CARB-AHERA TEM 0.001 s/cc Ana Sensitivity			
	PLM (asbestos)	A name (arrany	(ICP/MS (Lead)			
Sample Media		0.45 0.8 micron	MCEF Bulk Water Wipe			
SULTS DUE:	3 DAYS	AM / PM				
IAIN OF CUSTODY	DATA	plac submitted by DL	1/1 1:00 P			
Sending Info		pies submitted by	on <u>4 / 4</u> at ot 1			
Received by Lab:		ples received by				
Received by Analyst:	sam	ples received by				
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	AB (delete items	not applicable AND circle item	s applicable):			
1. Pickup requested:			Time of Call:			
Contact:	owledge receipt	of samples.				
a to have seen by her	DCM only		a contact project mensorar			
4. Analyze inside sar	nples by PCM	first; if any sample >0.01 f/c				
6 Analyze inside san	nples only; stop	with items 6, 7 or 8, as noted. if Avg >70 str/mm^2, contact	et PM before analyzing outsides or blanks			
7. Analyze all samples,	, including outsid	de samples and blanks.				
8. Do NOT analyze ou	tside or blank sa	mples.	result.	1		
10 Serial analysis; sl	top at first positi	sample with the highest PCM tive (>1%); first trace (<0.1%)	); except sheetrock and plaster samples.			
faile a Hould an	mainter unlose of	horwise indicated.		át.		
12. PCB: 1 PPM detection	n limit required. A L only analyze f	or REGULATED ASBESTOS.	up and Soxhlet extraction to meet the detection lim			
13. For AHERA TEN	t, only analyse i					
			Qty			
Report Number:		Supplies /Equipment	Qiy			
		Hi-Vol (3040)				
		Lo-Vol (3020)				
Invoice Number:		TEM / Pb cassettes (3520)	)			
		PCM cassettes (3500)	1			
		D. B. soundling sumply (27				

**PCB Laboratory Report** 



McCampbell Analytical, Inc.

"When Quality Counts"

## **Analytical Report**

WorkOrder:	1904245
Report Created for:	SCA Environmental, Inc.
	1 Lakeside Drive, Suite 215 Oakland, CA 94612
Project Contact: Project P.O.:	Dan Leung
Project:	B12936; WJE MT UMUNHUM RADAR TOWER SVY
<b>Project Received:</b>	04/04/2019

Analytical Report reviewed & approved for release on 04/10/2019 by:

Ja Coo

Yen Cao Project Manager

The report shall not be reproduced except in full, without the written approval of the laboratory. The analytical results relate only to the items tested. Results reported conform to the most current NELAP standards, where applicable, unless otherwise stated in the case narrative.



1534 Willow Pass Rd. Pittsburg, CA 94565 ♦ TEL: (877) 252-9262 ♦ FAX: (925) 252-9269 ♦ www.mccampbell.com CA ELAP 1644 ♦ NELAP 4033 ORELAP



## **Glossary of Terms & Qualifier Definitions**

Client:SCA Environmental, Inc.Project:B12936; WJE MT UMUNHUM RADAR TOWER SVYWorkOrder:1904245

#### **Glossary Abbreviation**

%D	Serial Dilution Percent Difference
95% Interval	95% Confident Interval
DF	Dilution Factor
DI WET	(DISTLC) Waste Extraction Test using DI water
DISS	Dissolved (direct analysis of 0.45 µm filtered and acidified water sample)
DLT	Dilution Test (Serial Dilution)
DUP	Duplicate
EDL	Estimated Detection Limit
ERS	External reference sample. Second source calibration verification.
ITEF	International Toxicity Equivalence Factor
LCS	Laboratory Control Sample
MB	Method Blank
MB % Rec	% Recovery of Surrogate in Method Blank, if applicable
MDL	Method Detection Limit
ML	Minimum Level of Quantitation
MS	Matrix Spike
MSD	Matrix Spike Duplicate
N/A	Not Applicable
ND	Not detected at or above the indicated MDL or RL
NR	Data Not Reported due to matrix interference or insufficient sample amount.
PDS	Post Digestion Spike
PDSD	Post Digestion Spike Duplicate
PF	Prep Factor
RD	Relative Difference
RL	Reporting Limit (The RL is the lowest calibration standard in a multipoint calibration.)
RPD	Relative Percent Deviation
RRT	Relative Retention Time
SPK Val	Spike Value
SPKRef Val	Spike Reference Value
SPLP	Synthetic Precipitation Leachate Procedure
ST	Sorbent Tube
TCLP	Toxicity Characteristic Leachate Procedure
TEQ	Toxicity Equivalents
TZA	TimeZone Net Adjustment for sample collected outside of MAI's UTC.
WET (STLC)	Waste Extraction Test (Soluble Threshold Limit Concentration)



a4

## **Glossary of Terms & Qualifier Definitions**

Client: SCA Environmental, Inc.

**Project:** B12936; WJE MT UMUNHUM RADAR TOWER SVY

**WorkOrder:** 1904245

#### **Analytical Qualifiers**

- A The reported value is determined using a "single point" calibration by GC-ECD as allowed by the method.
  - Reporting limits raised due to the sample's matrix prohibiting a full volume extraction.
- h4 Sulfuric acid permanganate (EPA 3665) cleanup.



1534 Willow Pass Road, Pittsburg, CA 94565-1701 Toll Free Telephone: (877) 252-9262 / Fax: (925) 252-9269 http://www.mccampbell.com / E-mail: main@mccampbell.com

## **Analytical Report**

Client:	SCA Environmental, Inc.
Date Received:	4/4/19 14:25
Date Prepared:	4/4/19
Project:	B12936; WJE MT UMUNHUM RADAR TOWER SVY

 WorkOrder:
 1904245

 Extraction Method:
 SW3540C/3630C

 Analytical Method:
 SW8082

 Unit:
 mg/kg

#### Polychlorinated Biphenyls (PCBs) Aroclors w/ Soxhlet Extraction and Silica Gel Clean-up

Client ID	Lab ID	Matrix	Date Colle	ected	Instrument	Batch ID	
CAULK- 12	1904245-001A Solid		04/03/2019		GC23 04081918.d	175749	
Analytes	Result	<u>Qualifiers</u>	<u>RL</u>	<u>DF</u>		Date Analyzed	
Aroclor1016	ND		0.50	1		04/08/2019 13:59	
Aroclor1221	ND		0.50	1		04/08/2019 13:59	
Aroclor1232	ND		0.50	1		04/08/2019 13:59	
Aroclor1242	ND		0.50	1		04/08/2019 13:59	
Aroclor1248	ND		0.50	1		04/08/2019 13:59	
Aroclor1254	1.3	А	0.50	1		04/08/2019 13:59	
Aroclor1260	ND		0.50	1		04/08/2019 13:59	
PCBs, total	1.3		0.50	1		04/08/2019 13:59	
<u>Surrogates</u>	<u>REC (%)</u>		<u>Limits</u>				
Decachlorobiphenyl	94		70-130			04/08/2019 13:59	
<u>Analyst(s):</u> LT			Analytical Com	ments: a4	l,h4		

Client ID	Lab ID	Matrix	Date Collected		Instrument	Batch ID
CAULK- 18	1904245-002A	Solid	04/03/2019		GC23 04081919.d	175749
Analytes	<u>Result</u>		<u>RL</u>	<u>DF</u>		Date Analyzed
Aroclor1016	ND		0.50	1		04/08/2019 14:14
Aroclor1221	ND		0.50	1		04/08/2019 14:14
Aroclor1232	ND		0.50	1		04/08/2019 14:14
Aroclor1242	ND		0.50	1		04/08/2019 14:14
Aroclor1248	ND		0.50	1		04/08/2019 14:14
Aroclor1254	ND		0.50	1		04/08/2019 14:14
Aroclor1260	ND		0.50	1		04/08/2019 14:14
PCBs, total	ND		0.50	1		04/08/2019 14:14
Surrogates	<u>REC (%)</u>		<u>Limits</u>			
Decachlorobiphenyl	98		70-130			04/08/2019 14:14
Analyst(s): LT			Analytical Com	<u>ments:</u> a4	l,h4	



## **Quality Control Report**

Client:	SCA Environmental, Inc.
Date Prepared:	4/4/19
Date Analyzed:	4/4/19 - 4/5/19
Instrument:	GC23
Matrix:	Solid
Project:	B12936; WJE MT UMUNHUM RADAR TOWER SVY

WorkOrder:	1904245
BatchID:	175749
<b>Extraction Method:</b>	SW3540C/3630C
Analytical Method:	SW8082
Unit:	mg/kg
Sample ID:	MB/LCS/LCSD-175749

### QC Summary for SW8082

Analyte	MB Result		MDL	RL		SPK Val	MB SS %REC		MB SS Limits
Aroclor1016	ND		0.050	0.050		-	-		
Aroclor1221	ND		0.050	0.050		-	-		-
Aroclor1232	ND		0.050	0.050		-	-		
Aroclor1242	ND		0.050	0.050		-	-		
Aroclor1248	ND		0.050	0.050		-	-		
Aroclor1254	ND		0.050	0.050		-	-		
Aroclor1260	ND		0.050	0.050		-	-		
PCBs, total	ND		N/A	0.050		-	-		-
Surrogate Recovery									
Decachlorobiphenyl	0.045					0.050	91		70-130
Analyte	LCS Result	LCSD Result	SPK Val		LCS %REC	LCSD %REC	LCS/LCSD Limits	RPD	RPD Limit
Aroclor1016	0.12	0.11	0.15		78	76	70-130	2.67	20
Aroclor1260	0.12	0.13	0.15		81	89	70-130	9.40	20
Surrogate Recovery									
Decachlorobiphenyl	0.051	0.053	0.050		102	105	70-130	3.43	20

Page	1	of	1
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McCampbell Analytical	, Inc.			CH	AIP	<b>1-0F</b> -	CUS	TODY	<b>RE</b> (	CORD	I	Page 1	of 1	i
Pittsburg, CA 94565-1701				Wor	kOrde	er: 19042	245	ClientC	ode:	SCAO				
(925) 252-9262	WaterTra	xWriteOn	EDF	E	xcel	E	QuIS	Email		HardCopy	ThirdPa	ty	_J-flaថ	J
					Detectio	on Summa	ry	Dry-Weig	ht					
Report to:					В	ill to:				Rec	quested TAT:	1(	) days;	
Dan Leung SCA Environmental, Inc. 1 Lakeside Drive, Suite 215 Oakland, CA 94612 415-378-4188 FAX: (510) 839- 620	Email: cc/3rd Party: PO: Project: 0		viro.com; labrepo IT UMUNHUM RA		)gm	SCA En 1 Lakes Oakland	ts Payabl vironmen ide Drive I, CA 946 io@scae	ntal, Inc. , Suite 215 612			te Received: te Logged:		)4/04/2( )4/04/2(	* = *
								Requested -	Tests (	See legend	below)			
Lab ID Client ID		Matrix	Collection Date	Hold	1	2	3	4 5	6	7	89	10	11	12
1904245-001 CAULK- 12	2	Solid	4/3/2019 00:00		А									
1904245-002 CAULK- 1	8	Solid	4/3/2019 00:00		А									

#### Test Legend:

1	8082_Soxhlet_SG_Solid
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9	

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ĺ	6	
	10	

3	
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11	

4	
8	
12	

#### Project Manager: Angela Rydelius

**Prepared by: Tina Perez** 

#### **Comments:**

NOTE: Soil samples are discarded 60 days after results are reported unless other arrangements are made (Water samples are 30 days). Hazardous samples will be returned to client or disposed of at client expense.

"When Quality Counts"

### WORK ORDER SUMMARY

Client Name Client Conta		RONMENTAL, INC	2.	Projec	et: B12936;	WJE MT UMUNHUM	I RADAR TC	WER SVY		k Order: C Level:	1904245 LEVEL 2
Contact's Er	nail: dleung@sca	a-enviro.com; labrep	orts99@gmail.	com Comn	nents:				Date	Logged:	4/4/2019
		WaterTrax	WriteOn	EDF	Excel	EQuIS Email		ppy ThirdParty	/J·	flag	
Lab ID	Client ID	Matrix	Test Name		Containers /Composites	Bottle & Preservative	De- chlorinated	Collection Date & Time	TAT	Sediment Content	Hold SubOut
1904245-001A	CAULK-12	Solid	SW8082 (PCE SG CU)	ss w/ Soxhlet Extraction	& 1	100ml White Cap		4/3/2019	5 days		
1904245-002A	CAULK- 18	Solid	SW8082 (PCE SG CU)	ss w/ Soxhlet Extraction	& 1	100ml White Cap		4/3/2019	5 days		

NOTES: - STLC and TCLP extractions require 2 days to complete; therefore, all TATs begin after the extraction is completed (i.e., One-day TAT yields results in 3 days from sample submission).

- MAI assumes that all material present in the provided sampling container is considered part of the sample - MAI does not exclude any material from the sample prior to sample preparation unless requested in writing by the client.

																_													10	104	24	15			
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Tele: ( 415 ) 867	7-9544				E-I	Mail	: dle	ung	asca	a-env	viro.	con	n			LBE		5520												tals					
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SAMPLE ID	Location/ Field Point Name	Date	Time	# Containers	Ground Water	Waste Water	Drinking Water	Sea Water	Soil	Air	Sludge	Other	HCL	HNO,	Other	BTEX & TPH as (	TPH as Diesel (8015)	Total Petroleum Oil & Grease (1664/5520 E/B&F)	Total Petroleum Hydrocarbons (418.1)	EPA 505/ 608 / 8081 (CI Pesticides)	EPA 608 / 8082 PCB's ; Aroclors only	EPA 507 / 8141 (N	EPA 515 / 8151 (Acidic Cl Herbicides)	EPA 524.2 / 624 / 8260 (VOCs)	EPA 525.2 / 625 / 8270 (SVOCs)	EPA 8270 SIM / 8	CAM 17 Metals (200.8 / 6020)***	LUFT 5 Metals (200.8 / 6020)***	Metals (200.8 / 6020)***	Lab to Filter samp analysis	PC8				
CAULK - 12		4/3	1	1		-			-	-		Х	-				$\vdash$	-	-				$\vdash$				-		$\vdash$	$\vdash$	×	$\vdash$	$\vdash$	$\vdash$	
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handling by MAI staff. N	Non-disclosure	incurs an	immediate	e \$250	surch	arge	and th	Aclie	nt is s	ubject	to fu	lleg	al lia	bility f	or ha	rm su	ffered	. Thar	k you	for yo	our un	dersto	Inding	Land	for all	owing	us to	work	safely					_	
*** If metals are reques	ted for water s	amples a	nd the wat	er typ				nthe	chair	ofcu	stody	, the	n MA				netals	by E2	00.8.	-	/	/										-			>
Relinquished By: Dan Leung		Date:	Time	19 A.	Reco	iver	By:		/						CE/ť		ONDI	TION			(		PCR	e• 1 m	nm d		COM on lit			ed Ar	athor	ized	to perf	orm	
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														1							P													Page	<u>: 8 c</u>



1534 Willow Pass Road, Pittsburg, CA 94565-1701 Toll Free Telephone: (877) 252-9262 / Fax: (925) 252-9269 http://www.mccampbell.com / E-mail: main@mccampbell.com

## Sample Receipt Checklist

Client Name:	SCA Environmental,				Date and Time Received	
Project:	B12936; WJE MT UI	MUNHUM RADAR TOWE	R SVY		Date Logged:	4/4/2019 Ting Darga
WorkOrder №:	1904245	Matrix: Solid			Received by: Logged by:	Tina Perez Tina Perez
Carrier:	<u>Client Drop-In</u>					
		Chain of	Custody	y (COC) Infor	mation	
Chain of custody	present?		Yes	✓	No 🗌	
Chain of custody	signed when relinquis	hed and received?	Yes	✓	No 🗌	
Chain of custody	agrees with sample la	bels?	Yes	✓	No 🗌	
Sample IDs noted	d by Client on COC?		Yes	✓	No 🗌	
Date and Time of	f collection noted by C	lient on COC?	Yes	$\checkmark$	No 🗌	
Sampler's name	noted on COC?		Yes	$\checkmark$	No 🗌	
COC agrees with	Quote?		Yes		No 🗌	NA 🗹
		Sam	ple Rece	eipt Informat	ion	
Custody seals int	tact on shipping contai	ner/cooler?	Yes		No 🗌	NA 🗹
Shipping containe	er/cooler in good condi	ition?	Yes	✓	No 🗌	
Samples in prope	er containers/bottles?		Yes	✓	No 🗌	
Sample containe	rs intact?		Yes	✓	No 🗌	
Sufficient sample	e volume for indicated t	rest?	Yes	✓	No 🗌	
		Sample Preserva	tion and	Hold Time (	HT) Information	
All samples recei	ived within holding time	e?	Yes		No 🗌	
Samples Receive	ed on Ice?		Yes		No 🗹	
Sample/Temp Bla	ank tomporaturo			Temp:		NA 🗸
			Voo		Νο	
	s have zero headspace		Yes Yes			
·		Nitrate 353.2/4500NO3:	Yes		No 🗌	NA
<pre>&lt;2; 522: &lt;4; 218.</pre>		Millale 333.2/4300NO3.	165			
UCMR Samples:						
	acceptable upon recei 3; 544: <6.5 & 7.5)?	ot (200.8: ≤2; 525.3: ≤4;	Yes		No 🗌	NA 🗹
Free Chlorine to	ested and acceptable	upon receipt (<0.1mg/L)?	Yes		No 🗌	NA 🗹



Mount Umunhum Radar Tower Condition Assessment and Recommendations November 13, 2019 Page B

# APPENDIX B: OPINION OF PROBABLE CONSTRUCTION COST BY HATTIN CONSTRUCTION MANAGEMENT

## MIDPENINSULA REGIONAL OPEN SPACE DISTRICT MT UMUNHUM RADAR TOWER REPAIR Los Gatos, CA

Opinion of Probable Construction Cost Conceptual Cost Estimate

Prepared for : Wiss Janney Elstner Associates, Inc.

July 11, 2019

by:

### HATTIN CONSTRUCTION MANAGEMENT, INC.

300 Frank H. Ogawa Plaza, Suite 239 Oakland, CA 94102 Telephone: (510) 832-5800 Fax: (510) 832-5900 www.hattincm.com

#### MIDPENINSULA REGIONAL OPEN SPACE DISTRICT MOUNT UMUNHUM RADAR TOWER REPAIR Los Gatos, CA

#### ESTIMATE OF PROBABLE CONSTRUCTION COST

#### **EXECUTIVE SUMMARY**

#### Introduction:

This Conceptual Design Cost Estimate represents the probable construction cost of **Midpeninsula Regional Open Space District – Mount Umunhum Radar Tower Repair, Los Gatos, CA**. Considering that the drawings are preliminary design submittal, certain components, which may be required as part of this project may not be shown or mentioned in this estimate. Allowances have been made when detail description of equipment, work definition, or quantities are not available. Material pricing and labor costs are obtained from historical cost data and similar projects. Mechanical and electrical costs are based similar projects. The unit costs include material, labor, and subcontractor's markup, and are based on the design level of documents received.

#### **Project Descriptions:**

Repair of Radar Tower in Mount Umunhum, Los Gatos, CA – The scope includes several Options.

#### **Documents Received as a Basis of Cost Estimate:**

The following documentation was used in preparation of this estimate:

• Preliminary Drawings S1 thru S6.

#### **Exclusions:**

The following items are excluded:

- Change Order Contingency
- Land Cost
- Cost of money
- Offsite Utilities & Connection Fees
- Professional Consultants' and Construction Management fees
- Administrative costs
- Fees for testing construction materials
- Plan checks and inspection
- Permits
- Legal and financing costs
- Furnishings, furniture, and equipment (FFE)
- Relocation costs, if required
- Contractor off-hours and compressed time work schedule, if required
- Escalation beyond that stated.
- ♦ LEED

#### **Possible Additional Cost Items:**

Items that may change the Estimate of Probable Construction Cost include, but are not limited to, the following:

- Modifications to the scope of work, drawings, specifications included in this estimate
- Unforeseen conditions
- Construction phasing requirements

- Excessive contract and general conditions, and restrictive technical specifications
- Equipment, material, systems or product that cannot be obtained from at least three different sources
- Delays beyond the projected schedule
- Any other non-competitive bid situations
- Any addenda, changes not included in the basis of estimates.

#### **Escalation:**

Escalation of 6% up to midpoint of construction is included in the estimate, assumed at 18 months from June 16, 2019 at the rate of 4% per annum.

#### ESTIMATING ASSUMPTIONS AND COMMENTS

#### General:

- a. Material prices are at 2nd Quarter 2019 level; include taxes and contractor's markups.
- b. Labor cost is based on prevailing wages.
- c. Work to be done during normal business hours.
- d. This estimate can vary due to change in scope.
- e. Quantities were obtained as shown on the drawings.
- f. Allowances are provided for items not shown in the drawings and are anticipated to be part of the estimate.
- g. Installation cost, supervision, and coordination for material and equipment are included in the estimate.
- h. General conditions assumed at 20% include mobilization, insurance, office personnel costs, dust control, and other items not mentioned in General requirements.
- i. Design Contingency/Estimating Contingency is assumed at 25% due to the level of drawings used in the estimate.

#### ESTIMATE OF PROBABLE CONSTRUCTION COST

The estimated Probable Construction Costs reflects the anticipated cost of the **MROSD Mount Umunhum Radar Radio Tower in Los Gatos, CA.** This estimate is based on a competitive open bid process with a recommended five or more bids from reputable general contractors, and a minimum of three bids for all subcontracted items.

Cost of materials, labor, equipment or services furnished by others, and the contractors' or vendors' methods of determining prices are determined by market and/or economic conditions. Hence, the Estimator cannot and does not guarantee that proposals, bids or actual project costs will not vary from this Estimate of Probable Construction Cost.

This Estimate of Probable Construction Cost is exclusive of all costs associated with changes, modifications or addenda to the drawings and/or specifications subsequent to the preparation of this estimate.

## Hattin Construction Management, Inc. Project and Construction Management Services

300 Frank H. Ogawa Plaza, Suite 239 Oakland, CA 94102 Telephone: (510)832-5800 - Fax: (510)832-5900

#### SUMMARY OF PROBABLE CONSTRUCTION COST

MIDPENINSULA REGIONAL OPEN SPACE DISTRICT MOUNT UMUNHUM RADAR TOWER REPAIR

Gross Area (SF) HCM Job Number: 2019-047

#### Los Gatos, CA

Type of Estimate: CONCEPTUAL ESTIMATE

Lead Estimator: EEV Date: 6/17/2019

				Revised: 7/11/2019
ITEM	DESCRIPTION		TOTAL	%
		AREA (SF)		
1	SITEWORK & STAIRS	\$	38,538	
2	CONCRETE			
	WALL CONCRETE REPAIR - BASE	\$	301,936	
	WALL CONCRETE REPAIR - OPTION 1	\$	698,603	
	WALL CONCRETE REPAIR - OPTION 2	\$	1,112,564	
3	THERMAL & MOISTURE PROTECTION			
	ROOF REPAIR - BASE	\$	210,588	
	SHORT TERM REPAIR	\$	114,318	
	LONG TERM REPAIR - OPTION 1	\$	255,120	
	LONG TERM REPAIR - OPTION 2	\$	491,698	
	<b>ROOF FALL PROTECTION (GUARDRAIL) - OPTION 1</b>	\$	89,866	
	<b>ROOF FALL PROTECTION - OPTION 2</b>	\$	35,424	
	WALL OPENING REPAIR - OPTION 1	\$	39,535	
	WALL OPENING REPAIR - OPTION 2	\$	60,631	
	WALL OPENING REPAIR - OPTION 3	\$	83,116	

	NINSULA REGIONAL OPEN SPACE DISTRICT						Estimate:		onceptual
	T UMUNHUM RADAR TOWER REPAIR				HC	CMJ	lob Number:		019-047
	atos, CA						Date:	6	/17/2019
SITEW	ORK & STAIRS						Revised:		/11/2019
Hattin C	onstruction Management, Inc.	1	AREA :	SF			Estimator:	E	EV/ARB
Div.	Description	Qty		Unit	Cost		Extension		Total
Descrip	tion: Replace (E) Bollards, Exterior Steel Doors, & Stair Handra	ails							
01	General Requirements Included in the General Conditions below.								
	General Requirements							\$	-
02	Existing Conditions Sitework & Interior Repairs								
	Remove two (2) existing bollards & replace w/ foldable bollards		2	EA	\$ 1,650.00	\$	3,300		
	Remove & replace single steel door on east side @ 1st floor level Remove & replace single steel door on west side @ 1st		1	EA	\$ 1,535.00	\$	1,535		
	floor level		1	EA	\$ 1,535.00	\$	1,535		
	Remove & replace steel handrail from 1st floor level to 5th floor levels		108	LF	\$ 100.00	\$	10,800		
	Infill openings w/ 12" x 24" metal grating		14	EA	\$ 250.00	\$	3,500		
	Existing Conditions							\$	20,670
	TOTAL DIRECT COST							\$	20,670
	General Conditions/General Requirements		20.0%						\$4,134
	SUBTOTAL General Contractor's Overhead & Profit		10.0%					\$	<b>24,804</b> \$2,480
	SUBTOTAL							\$	27,284
	Historic Preservation Factor		5.0%						\$1,364
	Design Contingency/Estimating Contingency SUBTOTAL		25.0%					\$	\$6,821
	Escalation up to midpoint of construction (18 months from June 12,							¢	35,470
	2019 @ 4%/year)		6.0%						\$2,128
	SUBTOTAL							\$	37,598
	Bonds TOTAL PROBABLE BID DAY CONSTRUCTION COST -		2.5%						\$940
	SITEWORK & STAIRS							\$	38,538

MOUNT UMUNHUM RADAR TOWER REPAIRHCM Job Number:Los Gatos, CADate:WALL CONCRETE REPAIR - BASERevised:	Conceptual 2019-047 6/17/2019 7/11/2019 EEV/ARB Total
Los Gatos, CA       Date:       Oute:       Date:       Oute:       Oute:	6/17/2019 7/11/2019 EEV/ARB
WALL CONCRETE REPAIR - BASE       Revised: 1         Hattin Construction Management, Inc.       AREA : SF       Estimator: 1         Div.       Description       Qty       Unit       Cost       Extension         Description:       Repair large cracks, voids, spalls, & delaminations at pilasters, the thickened roof slab edge along tops of the walls, & selected portions       Of the construction of the walls, & selected portions       Of the construction of the walls, & selected portions       Of the construction of the walls, & selected portions       Description:       Repair large cracks, voids, spalls, & delaminations at pilasters, the thickened roof slab edge along tops of the walls, & selected portions       Cost       Extension         01       General Requirements       General Requirements       General Requirements       General Requirements	7/11/2019 EEV/ARB
Hattin Construction Management, Inc.       AREA : SF       Estimator:         Div.       Description       Qty       Unit       Cost       Extension         Description:       Repair large cracks, voids, spalls, & delaminations at pilasters, the thickened roof slab edge along tops of the walls, & selected portions       01       General Requirements	EEV/ARB
Div.       Description       Qty       Unit       Cost       Extension         Description:       Repair large cracks, voids, spalls, & delaminations at pilasters, the thickened roof slab edge along tops of the walls, & selected portions       01       General Requirements	
Description:       Repair large cracks, voids, spalls, & delaminations at pilasters, the thickened roof slab edge along tops of the walls, & selected portions         01       General Requirements	Total
thickened roof slab edge along tops of the walls, & selected portions 01 General Requirements	
General Requirements \$	•
03 CONCRETE	
Remove loose or delaminated concrete & sawcut the	
perimeter of the repair area, assume all pilaster edges require repair 2,028 LF \$ 35.00 70,980	
Clean the corrosion & apply a protective coating to the exposed steel, allow 1,582 SF \$ 7.50 11,865	
Patch the concrete spalls using an appropriate repair mortar, allow 1,582 SF \$ 50.00 79,100	
CONCRETE \$	161,945
CONCRETE \$	101,940
TOTAL DIRECT COST \$	161,945
General Conditions/General Requirements 20.0%	\$32,389
SUBTOTAL \$	194,334
General Contractor's Overhead & Profit 10.0%	\$19,433
SUBTOTAL \$ Historic Preservation Factor 5.0%	<b>213,767</b> \$10,688
Design Contingency/Estimating Contingency 25.0%	\$53,442
SUBTOTAL	277,898
Escalation up to midpoint of construction (18 months from June 12,	
2019 @ 4%/year) 6.0%	\$16,674
SUBTOTAL \$	294,571
	\$7,364
TOTAL PROBABLE BID DAY CONSTRUCTION COST -WALL CONCRETE REPAIRS - BASE \$	301,936

							-	
	NINSULA REGIONAL OPEN SPACE DISTRICT					Estimate:		onceptual
	T UMUNHUM RADAR TOWER REPAIR				HCM	I Job Number:		019-047
	atos, CA					Date:		/17/2019
	CONCRETE REPAIR - OPTION 1					Revised:	7	/11/2019
Hattin C	onstruction Management, Inc.	AREA :	SF			Estimator:	E	EV/ARB
Div.	Description	Qty	Unit		Cost	Extension		Total
Descript	tion: Patch & Coat; Repair the surface of the exterior concrete between pilasters to be suitable to receive an elastomeric							
01	General Requirements Included in the General Conditions below.							
	General Requirements						\$	-
03	CONCRETE Repair shallow rough & poorly consolidated concrete at walks Remove loose concrete & roughen surface to receive concrete repair mortar, allow 20% of total wall area Surface patch concrete for smooth, well bonded finish Coat concrete walls & pilasters w/ elastomeric coating	s using the fo 7,320 7,320 23,700	SF SF SF SF	ng pro \$ \$ \$	00000000000000000000000000000000000000	73,200 183,000 118,500		
	CONCRETE						\$	374,700
	TOTAL DIRECT COST						\$	374,700
	General Conditions/General Requirements	20.0%				-		\$74,940
	SUBTOTAL General Contractor's Overhead & Profit	10.0%					\$	449,640
	SUBTOTAL	10.0%				-	\$	\$44,964 <b>494,604</b>
	Historic Preservation Factor	5.0%					Ψ	\$24,730
	Design Contingency/Estimating Contingency	25.0%						\$123,651
	SUBTOTAL					-	\$	642,985
	Escalation up to midpoint of construction (18 months from June 12,							
	2019 @ 4%/year)	6.0%				-		\$38,579
	SUBTOTAL	0.50/					\$	681,564
	Bonds TOTAL PROBABLE BID DAY CONSTRUCTION COST -WALL	2.5%				-		\$17,039
	CONCRETE REPAIRS - OPTION 1						\$	698,603

	NINSULA REGIONAL OPEN SPACE DISTRICT				Estimate:	Conceptual
MOUN	T UMUNHUM RADAR TOWER REPAIR			HCM	Job Number:	2019-047
Los G	atos, CA				Date:	6/17/2019
WALL	CONCRETE REPAIR - OPTION 2				Revised:	7/11/2019
Hattin C	Construction Management, Inc.	AREA	: SF		Estimator:	EEV/ARB
Div.	Description	Qty	Unit	Cost	Extension	Total
Descrip	tion: Stucco Cladding between Pilasters					
01	General Requirements Included in the General Conditions below.					
	General Requirements					\$-
09	FINISHES					
	Installs mechanically attached 3-coat cement plaster (stucco (thickness of about ¾") on the walls between the concrete pi substrate for an elastomeric waterproofing coating Install mechanically fastened metal lath & 3-coat on the					
	exterior walls between pilasters.	18,930	SF	\$ 25.00	473,250	
	Install horizontal control joints at the bottom of each floor slab level	832	LF	\$ 5.00	4,160	
	Install intermediate horizontal control joints at the 4th & 5th floors	164	LF	\$ 5.00	820	
	Coat stucco & conc. pilasters w/ elastomeric coating	23,700	SF	\$ 5.00	118,500	
	FINISHES					\$ 596,730
	TOTAL DIRECT COST					\$ 596,730
	General Conditions/General Requirements	20.0%	, )			\$119,346
	SUBTOTAL				-	\$ 716,076
	General Contractor's Overhead & Profit	10.0%	, )		_	\$71,608
	SUBTOTAL	E 00/				\$ 787,684
	Historic Preservation Factor	5.0%				\$39,384
	Design Contingency/Estimating Contingency SUBTOTAL	25.0%	)		-	\$196,921 \$ 1,023,989
	Escalation up to midpoint of construction (18 months from June 12,					φ 1,023,707
	2019 @ 4%/year)	6.0%	, D			\$61,439
	SUBTOTAL				-	\$ 1,085,428
	Bonds	2.5%	, D		_	\$27,136
	TOTAL PROBABLE BID DAY CONSTRUCTION COST -WALL CONCRETE REPAIRS - OPTION 2				_	\$ 1,112,564

	NINSULA REGIONAL OPEN SPACE DISTRICT T UMUNHUM RADAR TOWER REPAIR				Ц	<u>^</u> M	Estimate: Job Number:		Conceptua 2019-047
	atos, CA				п	. ۱۷۱ر	Date:		6/17/2019
	REPAIR - BASE						Revised:		7/11/2019
	onstruction Management, Inc.	AREA :	SE				Estimator:		EEV/ARB
Div.	Description	Qty	Unit		Cost		Extension		Total
escript	•	Qly	Unit		0031		Extension	—	Total
01	General Requirements Included in the General Conditions below.								
	General Requirements							\$	-
07	Thermal & Moisture Protection								
	Remove and replace (e) roof drains & plumbing w/in the building to the storm drainage system on the east side of the building	4	EA	\$	1,000.00	\$	4,000		
	Remove aluminum sheet shielding from underside of the roof slab to observe the condition of the slab for locations of water intrusion & possible structural damage Inspect the structural slab soffit & repair damage, allow 25%	3,630 908	SF SF	\$ \$	5.00 100.00	\$ \$	18,150 90,800		
	Thermal & Moisture Protection							\$	112,9
	TOTAL DIRECT COST							\$	112,9
	General Conditions/General Requirements	20.0%						•	\$22,5
	SUBTOTAL							\$	135,5
	General Contractor's Overhead & Profit	10.0%							\$13,5
	SUBTOTAL Historic Preservation Factor	5.0%						\$	<b>149,0</b> \$7,4
	Design Contingency/Estimating Contingency	25.0%							\$37,2
	SUBTOTAL							\$	193,8
	Escalation up to midpoint of construction (18 months from June 12,								
	2019 @ 4%/year)	6.0%						<u> </u>	\$11,6
	SUBTOTAL	2 50/						\$	205,4 ¢ت 1
	Bonds TOTAL PROBABLE BID DAY CONSTRUCTION COST - ROOF	2.5%						—	\$5,1
	BASE REPAIR							\$	210,5

/IIDPE	NINSULA REGIONAL OPEN SPACE DISTRICT							Estimate:	С	onceptual
IOUN	T UMUNHUM RADAR TOWER REPAIR					HC	CM .	Job Number:	2	2019-047
	atos, CA							Date:	6	/17/2019
HOR	T TERM REPAIR							Revised:	7	/11/2019
attin C	onstruction Management, Inc.		AREA :	SF				Estimator:	E	EV/ARB
Div.	Description	Qty		Unit		Cost		Extension		Total
escrip	tion: Targeted Maintenance Repairs & Water Management									
01	General Requirements Included in the General Conditions below.									
	General Requirements								\$	-
07	Thermal & Moisture Protection									
	Short -Term Target Maintenance Repairs & Water Manager	nent								
	Perform targeted maintenance above areas where									
	leakage is occurring possibly w/ fluid-applied		3,630	SEDo	¢	10.00	¢	36,300		
	membranes, as appropriate, allow 100% of roof area Perform follow-up repairs, allow 50% of initial repair		,	LS		15,000.00		30,300 15,000		
	Re-seal expansion joints to encourage surface drainage & limit water penetration			LF	\$	10.00		940		
	Seal open gaps & joints around penetrations, pitch pockets, hatches, & other roof flop items across the entire roof, allow		3,630	SFRo	\$	2.50	\$	9,075		
	Thermal & Moisture Protection								\$	61,3
	TOTAL DIRECT COST								\$	61,3
	General Conditions/General Requirements		20.0%						Ψ	\$12,20
	SUBTOTAL								\$	73,5
	General Contractor's Overhead & Profit		10.0%							\$7,3
	SUBTOTAL Historic Preservation Factor		5.0%						\$	<b>80,9</b> 3 \$4,04
	Design Contingency/Estimating Contingency		25.0%							\$20,2
	SUBTOTAL								\$	105,2
	Escalation up to midpoint of construction (18 months from June 12,									
	2019 @ 4%/year) SUBTOTAL		6.0%						\$	\$6,3 <sup>°</sup> 111,5
	Bonds		2.5%						φ	\$2,78
	TOTAL PROBABLE BID DAY CONSTRUCTION COST - SHORT		2.070							Ψ_1
	TERM REPAIR									

	ENINSULA REGIONAL OPEN SPACE DISTRICT IT UMUNHUM RADAR TOWER REPAIR				HC	CM .	Estimate: Job Number:		onceptua 2019-047
	atos, CA						Date:		/17/2019
	TERM REPAIR - OPTION 1						Revised:	7	/11/2019
attin (	Construction Management, Inc.	AF	REA :	SF			Estimator:	E	EV/ARB
Div.	Description	Qty		Unit	Cost		Extension		Total
escrip	tion: Single- Ply Membrane over (N) Roof Membrane								
	Abandon the concrete and steel surface as membrane s install (n) light gauge metal framing & sheathing w/ a sir membrane that is adhered, or both mechanically fastene the sheathing.	ngle ply		d to					
01	General Requirements Included in the General Conditions below.								
	General Requirements							\$	-
07	Thermal & Moisture Protection Remove roof top penetrations & obstructions where possible	9							
	Detach (e) equipment that can be removed w/o damage Cut penetrating items flush with the top surface of the		1	EA	\$ 1,000.00	\$	1,000		
	wearing slab or concrete pedestal, allow Coordinate the height of new & existing rooftop items to	3	,630	SF	\$ 3.00	\$	10,890		
	remain w/ increased height of the (n) roof deck					\$	-		
	Remove the roof hatch & reconfigure roof access; replace roof hatch to accommodate the new (increased) height of the roof deck		2	EA	\$ 3,500.00	\$	7,000		
	Coordinate location of fall protection anchors & height guardrails w/ height of (n) roof deck		16	HRS	\$ 150.00	\$	2,400		
	Install light gauge metal framing w/ exterior sheathing to create a (n) sloped roof deck across the entire roof including above the concrete pedestal; ventilate the cavity between the sheathing & wearing slab	3	,630	SF	\$ 15.00	\$	54,450		
	Conceal remaining roofrop penetrations & obstructions w/in the (n) roof deck assembly or w/ box-outs where the penetrations or obstruction is taller than the level of the (n) roof deck (sheathing); protect (e) materials w/ a separation layer, (fill cavity w/ closed-cell spray foam insulation to minimize the risk of condensation w/in the								
	assembly	_	60	SF	\$ 20.00	\$	1,200		
	Install single-ply roofing membrane Slope (n) roofing to the location of (e) roof drains &	3	,630	SF	\$ 15.00	\$	54,450		
	integrate w/ (n) drains	3	,630	SF	\$ 1.50	\$	5,445		
	Thermal & Moisture Protection							\$	136,8
	TOTAL DIRECT COST	0	0.00/					\$	136,8
	General Conditions/General Requirements SUBTOTAL		0.0%					\$	\$27,3 <b>164</b> ,2
	General Contractor's Overhead & Profit SUBTOTAL		0.0%					\$	\$16,4 <b>180,6</b>
	Historic Preservation Factor Design Contingency/Estimating Contingency		5.0% 5.0%					¢	\$9,0 \$45,1
	SUBTOTAL Escalation up to midpoint of construction (18 months from June 12, 2019 @ 4%/year)		6.0%					\$	<b>234</b> ,8 \$14,0
	SUBTOTAL Bonds		2.5%					\$	<u>۵۱4,0</u> 248,8 \$6,2
									φU,2

	NINSULA REGIONAL OPEN SPACE DISTRICT T UMUNHUM RADAR TOWER REPAIR				H	CM .	Estimate: Job Number:		onceptual 019-047
Los Ga	atos, CA						Date:	6	/17/2019
ONG	TERM REPAIR - OPTION 2						Revised:	7.	/11/2019
Hattin C	onstruction Management, Inc.	AREA	: SF				Estimator:	E	EV/ARB
Div.	Description	Qty	Unit		Cost		Extension		Total
Descrip	tion: Buried Roofing Membrane								
01	Remove & replace the concrete wearing slab & original hot rubberized asphalt w/ a (n) concrete wearing slab pr General Requirements Included in the General Conditions below.		ofing w/						
	General Requirements							\$	-
07	Thermal & Moisture Protection								
07	Remove concrete wearing slab & underlying roofing assembly to expose then structure concrete slab	2,510	SF	\$	7.50	\$	18,825		
	Inspect the top of the structural slab & repair damage, allow	1	LS	\$ 15	,000.00	\$	15,000		
	Repair rough, delaminated, & spalled concrete on both horizontal & vertical surfaces at the concrete pedestal,	4 570	05	•		¢	105 ( 00		
	allow 50% of area for repair Remove roof top penetrations & obstructions where	1,570	SF	\$	80.00	\$	125,600		
	possible, allow	3,630	SF	\$	1.50	\$	5,445		
	Detach items that can be removed w/o damage	3	EA	\$	500.00	\$	1,500		
	Cut penetrating items flush with the top surface of the wearing slab or concrete pedestal, allow	3,630	SF	\$	3.00	\$	10,890		
	Prepare remaining rooftop penetrations & obstruction for integration into HRA membrane by sand blasting steel	3	EA	\$	500.00	\$	1,500		
	Install HRA membrane w/ (n) concrete wearing slab on horizontal roof surface	2,510	SF	\$	25.00	\$	62,750		
	Integrate HRA membrane w/ (n) roof drains in original locations, allow roof area	3,630	SF	\$	2.50	\$	9,075		
	Install elastomeric coating on vertical surface (wall) of concrete pedestal	450	SF	\$	5.00	\$	2,250		
	Remove corrosion from steel that is exposed above the roof & coat w/ high performance coating system, allow	3,630	SFRc	\$	3.00	\$	10,890		
							·		
	Thermal & Moisture Protection							\$	263,72
	TOTAL DIRECT COST							\$	263,72
	General Conditions/General Requirements SUBTOTAL	20.09	6					\$	\$52,74 <b>316,4</b> 7
	General Contractor's Overhead & Profit	10.09	%					Ψ	\$31,64
	SUBTOTAL							\$	348,11
	Historic Preservation Factor	5.0%							\$17,40
	Design Contingency/Estimating Contingency SUBTOTAL	25.09	0					\$	\$87,02 <b>452,5</b> 5
	Escalation up to midpoint of construction (18 months from June 12,							Ψ	102,00
	2019 @ 4%/year)	6.0%	6						\$27,15
	SUBTOTAL	2 50	V					\$	479,70
	Bonds TOTAL PROBABLE BID DAY CONSTRUCTION COST - LONG	2.5%	0						\$11,99
	TERM REPAIR OPTION 2							\$	491,69

	NINSULA REGIONAL OPEN SPACE DISTRICT				Estimate:	С	onceptual
MOUN	T UMUNHUM RADAR TOWER REPAIR			HC	CM Job Number:	2	019-047
Los Ga	atos, CA				Date:	6	/17/2019
ROOF	FALL PROTECTION REPAIRS - OPTION 1				Revised:	7.	/11/2019
Hattin C	onstruction Management, Inc.	AREA	: SF		Estimator:	E	EV/ARB
Div.	Description	Qty	Unit	Cost	Extension		Total
Descript	tion: Install a guardrail system around the entire roof						
01	General Requirements						
01	Included in the General Conditions below.		See G	eneral Conditic	ns/General Requ	uirem	ents below
	General Requirements					\$	-
07	Thermal & Moisture Protection						
0,	Install a guardrail system around the entire roof to match the original guardrail system configuration, but using more						
	durable materials than ther original wood framing	241	LF	\$ 200.00	\$ 48,200		
	Thermal & Moisture Protection					\$	48,200
						¢	40.000
	TOTAL DIRECT COST General Conditions/General Requirements	20.0%				\$	<b>48,200</b> \$9,640
	SUBTOTAL	20.07	)			\$	57,840
	General Contractor's Overhead & Profit	10.0%	,			Ψ	\$5,784
	SUBTOTAL		-			\$	63,624
	Historic Preservation Factor	5.0%	, D				\$3,181
	Design Contingency/Estimating Contingency	25.0%	D				\$15,906
	SUBTOTAL					\$	82,711
	Escalation up to midpoint of construction (18 months from June 12,						
	2019 @ 4%/year)	6.0%	)				\$4,963
	SUBTOTAL					\$	87,674
	Bonds	2.5%	þ				\$2,192
	TOTAL PROBABLE BID DAY CONSTRUCTION COST -ROOF FALL PROTECTION REPAIRS OPTION 1					\$	89,866

MIDPE	NINSULA REGIONAL OPEN SPACE DISTRICT						Estimate:	С	onceptual
MOUN	T UMUNHUM RADAR TOWER REPAIR				HC	CM .	Job Number:	2	019-047
Los Ga	atos, CA						Date:	6	/17/2019
ROOF	FALL PROTECTION REPAIRS - OPTION 2						Revised:	7	/11/2019
Hattin C	onstruction Management, Inc.	AF	REA :	SF			Estimator:	E	EV/ARB
Div.	Description	Qty		Unit	Cost		Extension		Total
Descrip	tion: Install dedicated rooftop fall arrest anchors meeting CAL requirement into the side of the concrete pedestal	JOSHA							
01	General Requirements Included in the General Conditions below.								
	General Requirements							\$	-
07	Thermal & Moisture Protection								
	Install dedicated rooftop fall arrest anchors meeting CAL/OSHA requirement into the side of the concrete pedestal; install anchors using a drill w/ built-in vacuum or use wet-drilling methods to contain amount of asbestos								
	traced from concrete pedestal Install CAL/OSHA compliant guardrail system around the		6	EA	\$ 1,500.00	\$	9,000		
	(e) roof hatch		40	LF	\$ 250.00	\$	10,000		
	Thermal & Moisture Protection							\$	19,000
	TOTAL DIRECT COST							\$	19,000
	General Conditions/General Requirements	2	0.0%					Ψ	\$3,800
	SUBTOTAL							\$	22,800
	General Contractor's Overhead & Profit	1	0.0%					<u> </u>	\$2,280
	SUBTOTAL Historic Preservation Factor		5.0%					\$	<b>25,080</b> \$1,254
	Design Contingency/Estimating Contingency		5.0%						\$1,234 \$6,270
	SUBTOTAL	2	0.070					\$	32,604
	Escalation up to midpoint of construction (18 months from June 12,								
	2019 @ 4%/year)		6.0%						\$1,956
	SUBTOTAL							\$	34,560
	Bonds		2.5%						\$864
	TOTAL PROBABLE BID DAY CONSTRUCTION COST -ROOF FALL PROTECTION REPAIRS OPTION 2							\$	35,424

MIDPE	NINSULA REGIONAL OPEN SPACE DISTRICT					Estimate:	Conceptual
MOUN	T UMUNHUM RADAR TOWER REPAIR				HCM J	ob Number:	2019-047
	atos, CA					Date:	6/17/2019
	OPENING REPAIRS - OPTION 1					Revised:	7/11/2019
Hattin C	onstruction Management, Inc.	ARE	A: \$	SF		Estimator:	EEV/ARB
Div.	Description	Qty	U	Init	Cost	Extension	Total
Descrip 01	tion: Maintain in (e) configuration. Maintain all wall openi current configuration & adds a system to collect & c that currently leaks through the opening General Requirements Included in the General Conditions below. General Requirements	-		er			\$-
07	Thermal & Moisture Protection						
07	Install a waterproofing membrane w/in the collection basin & up and over the perimeter curbs; extend the waterproofing through the beneath the opening closure plate including a metal flashing w/ a drip edge if possible	14	17 5	SF	15.00	2,205	
	Thermal & Moisture Protection						\$ 2,205
09	Finishes Remove corrosion & paint steel frames at openings	41	0 1	LF	10.00	4,100	
	Finishes						\$ 4,100
15	Mechanical						
15	Install water collection & drainage systems at each area of wall,openings where leakage occurs - assume 4" - 8" for most openings	14	10 I	LF	65.00	9,100	
	Extend collection basin a sufficient distance past opening jambs & towards the building interior to collect water; assume 2 - 4 feet for most opening	12	28 I	LF	25.00	3,200	
	Add drains & plumbing to the exterior storm drainage system on the east side of the building at the larger or more exposed locations, depending on the extent of actual water penetration at each opening, allow	6	5 I	LF	40.00	2,600	
	Mechanical						\$ 14,900
							¢ 01.00F
	TOTAL DIRECT COST General Conditions/General Requirements SUBTOTAL	20.0	)%			_	\$ 21,205 \$4,241 \$ 25,446
	General Contractor's Overhead & Profit	10.0	)%				\$2,545
	SUBTOTAL					_	\$ 27,991
	Historic Preservation Factor Design Contingency/Estimating Contingency	5.0 25.0					\$1,400 \$6,998
	SUBTOTAL	20.0	//0			-	\$ 36,388
	Escalation up to midpoint of construction (18 months from June 12,						
	2019 @ 4%/year)	6.0	)%			-	\$2,183 \$38,571
	SUBTOTAL Bonds	2.5	5%				<b>\$ 38,571</b> \$964
	TOTAL PROBABLE BID DAY CONSTRUCTION COST -WALL OPENING REPAIRS - OPTION 1	2.0				-	\$ 39,535

DUN	ENINSULA REGIONAL OPEN SPACE DISTRICT IT UMUNHUM RADAR TOWER REPAIR atos, CA			НСМ	Estimate: Job Number: Date:	Conceptua 2019-047 6/17/2019
ALL	OPENING REPAIRS - OPTION 2 Construction Management, Inc.	AREA	: SF		Revised: Estimator:	7/11/2019 EEV/ARB
Div.	Description	Qty	Unit	Cost	Extension	Total
scrip	tion: Selective Ventilation & Closure of Openings. Evaluate t requirements for the building and & install louvers wher ventilation, & replaces the steel plates at the other ope tight infills that do not necessarily match the historic app configuration of the building	e needed for nings w/ wat	r			
01	General Requirements Included in the General Conditions below.					
	General Requirements					\$-
05	Metals Remove all steel plates at wall openings	348	SF	5.00	1,740	
	Metals					\$ 1,7
08	Openings Install ventilation louvers w/ animal screens where needed for ventilation	118	SF	85.00	10,030	
	Openings					\$ 10,0
~~						
09	Finishes Infill openings that are not required for ventilation w/ concrete, light gauge metal framing and stucco, or steel plates to create watertight assemblies, assume light gauge framing & stucco	230	SF	25.00	5,750	
	Finishes					\$ 5,7
15	Mechanical					
15	Evaluate building ventilation & hazardous materials requirements to determine appropriate treatment for each wall & overall ventilation of the building, assume total exterior walls	100	HRS	150.00	15,000	
	Mechanical					\$ 15,0
	TOTAL DIRECT COST					\$ 32,5
	General Conditions/General Requirements	20.0%	)		_	\$6,5
	SUBTOTAL General Contractor's Overhead & Profit	10.0%	)			<b>\$ 39,0</b> \$3,9
	SUBTOTAL	F 00/			-	\$ 42,9
	Historic Preservation Factor Design Contingency/Estimating Contingency SUBTOTAL	5.0% 25.0%			-	\$2,1 \$10,7 <b>\$ 55,8</b>
	Escalation up to midpoint of construction (18 months from June 12, 2019 @ $4\%$ /year)	6.0%	)			\$3,3
	SUBTOTAL	<b>3</b> F0/			-	\$ 59,1
	Bonds TOTAL PROBABLE BID DAY CONSTRUCTION COST -WALL OPENING REPAIRS - OPTION 2	2.5%	)		-	\$1, <sup>2</sup> \$60,6

IIDPENINSULA REGIONAL OPEN SPACE DISTRICT IOUNT UMUNHUM RADAR TOWER REPAIR					Estimate: HCM Job Number:	
Los Gatos, CA NALL OPENING REPAIRS - OPTION 3				TICIV	Date:	2019-047 6/17/2019
					Revised:	7/11/2019
ttin C	construction Management, Inc.	AREA	: SF		Estimator:	EEV/ARB
Div.	Description	Qty	Unit	Cost	Extension	Total
scrip	tion:					
	Restore to original appearance. Similar to Option 2 exc sheet metal hoods, & aluminum windows are installed t general appearance & configuration of original opening: Figure 1, but upgrade the weather protection where fea	o match the s as shown i				
01	General Requirements Included in the General Conditions below.					
	General Requirements					\$ -
05	Metals					
	Remove all steel plates at wall openings; optionally (e) plates can remain where additional ventilation is not					
	needed where hoods are to be installed	230	SF	5.00	1,150	
	Metals					\$ 1,1
07	Thermal & Moisture Protection					
	Install sheet metal hoods at the louvered openings to					
	provide improved resistance to wind-driven rainventilation louvers w/ animal screens where needed for ventilation	44	LF	100.00	4,400	
	Thermal & Moisture Protection					\$ 4,4
08	Openings Install ventilation louvers w/ animal screens similar in appearance & configuration to original hoods, adapted to					
	meet the current ventilation requirements	118	SF	85.00	10,030	
	Openings					\$ 10,0
09	Finishes					
	Install (n) watertight permanent infill panels similar in appearance to the original hollow core metal panels to fully close off access openings that originally had removable hollow motel panels for equipment access	220	<u>ور</u>	E0.00	11 500	
	hollow metal panels for equipment access Finishes	230	SF	50.00	11,500	\$ 11,5
45	Madaatat					
15	Mechanical Evaluate building ventilation & hazardous materials					
	requirements to determine appropriate treatment for each					
	wall & overall ventilation of the building, assume total exterior walls	100	HRS	150.00	15,000	
	Coordinate locations for ventilation louvers & hoods w/	100	1110	100.00	10,000	
	configuration of historic openings, allow Mechanical	10	LOC	250.00	2,500	\$ 17,5
	TOTAL DIRECT COST					\$ 44,5
	General Conditions/General Requirements	20.0%	b D		-	\$8,9
	SUBTOTAL General Contractor's Overhead & Profit	10.0%	/ D			\$ 53,4 \$5,3
	SUBTOTAL	10.07	•		-	\$ 58,8
	Historic Preservation Factor	5.0%				\$2,9
	Design Contingency/Estimating Contingency	25.0%	<u>_</u>			\$14,7

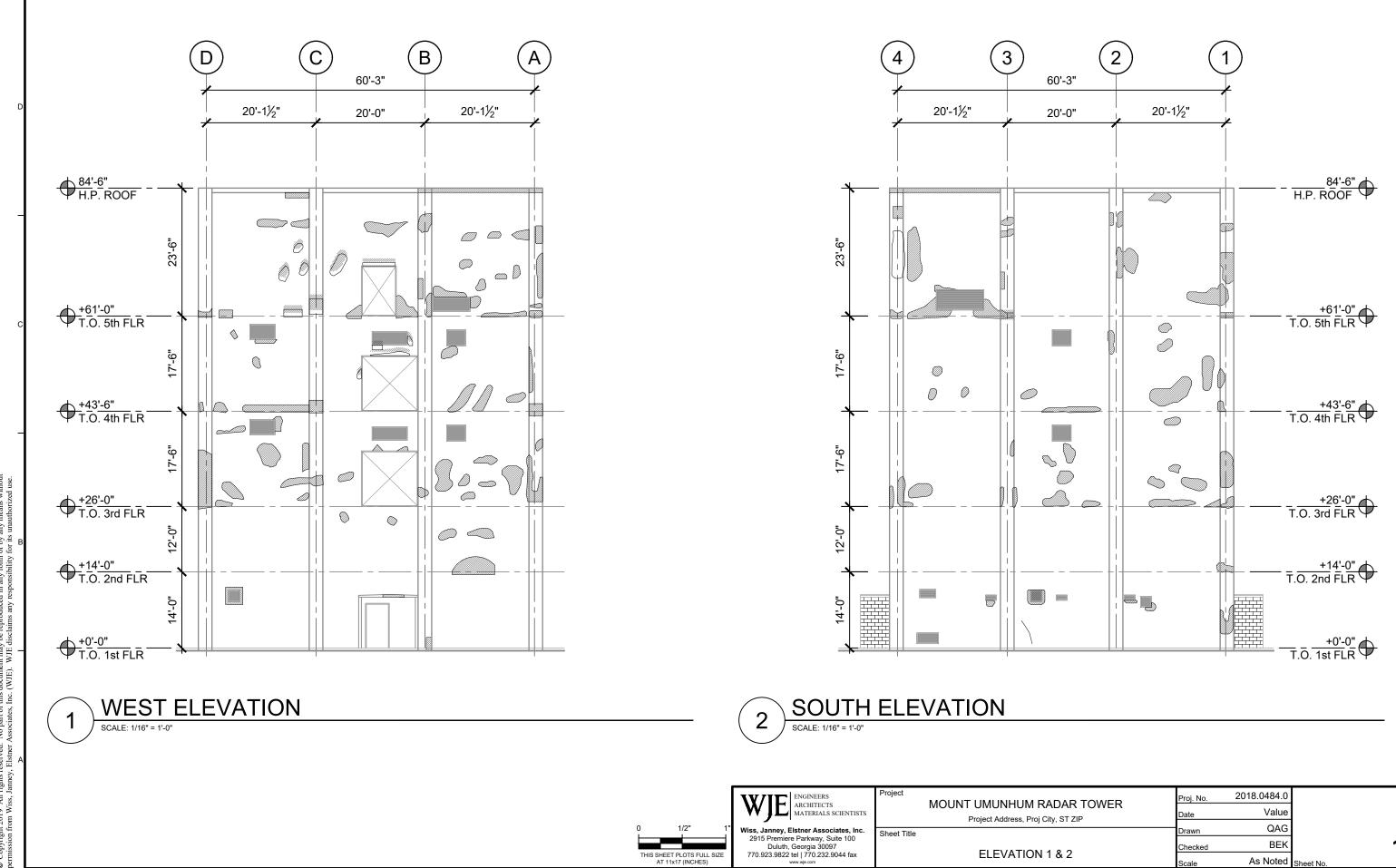
#### MIDPENINSULA REGIONAL OPEN SPACE DISTRICT Estimate: Conceptual MOUNT UMUNHUM RADAR TOWER REPAIR HCM Job Number: 2019-047 Los Gatos, CA Date: 6/17/2019 WALL OPENING REPAIRS - OPTION 3 Revised: 7/11/2019 Hattin Construction Management, Inc. AREA: SF EEV/ARB Estimator: Description Unit Extension Div. Qty Cost Total Description: Restore to original appearance. Similar to Option 2 except that louvers, sheet metal hoods, & aluminum windows are installed to match the general appearance & configuration of original openings as shown in Figure 1, but upgrade the weather protection where feasible Escalation up to midpoint of construction (18 months from June 12, 2019 @ 4%/year) 6.0% \$4,590 SUBTOTAL 81.089 \$ Bonds 2.5% \$2,027 TOTAL PROBABLE BID DAY CONSTRUCTION COST -WALL **OPENING REPAIRS - OPTION 3** \$ 83,116





Mount Umunhum Radar Tower Condition Assessment and Recommendations November 13, 2019 Page C

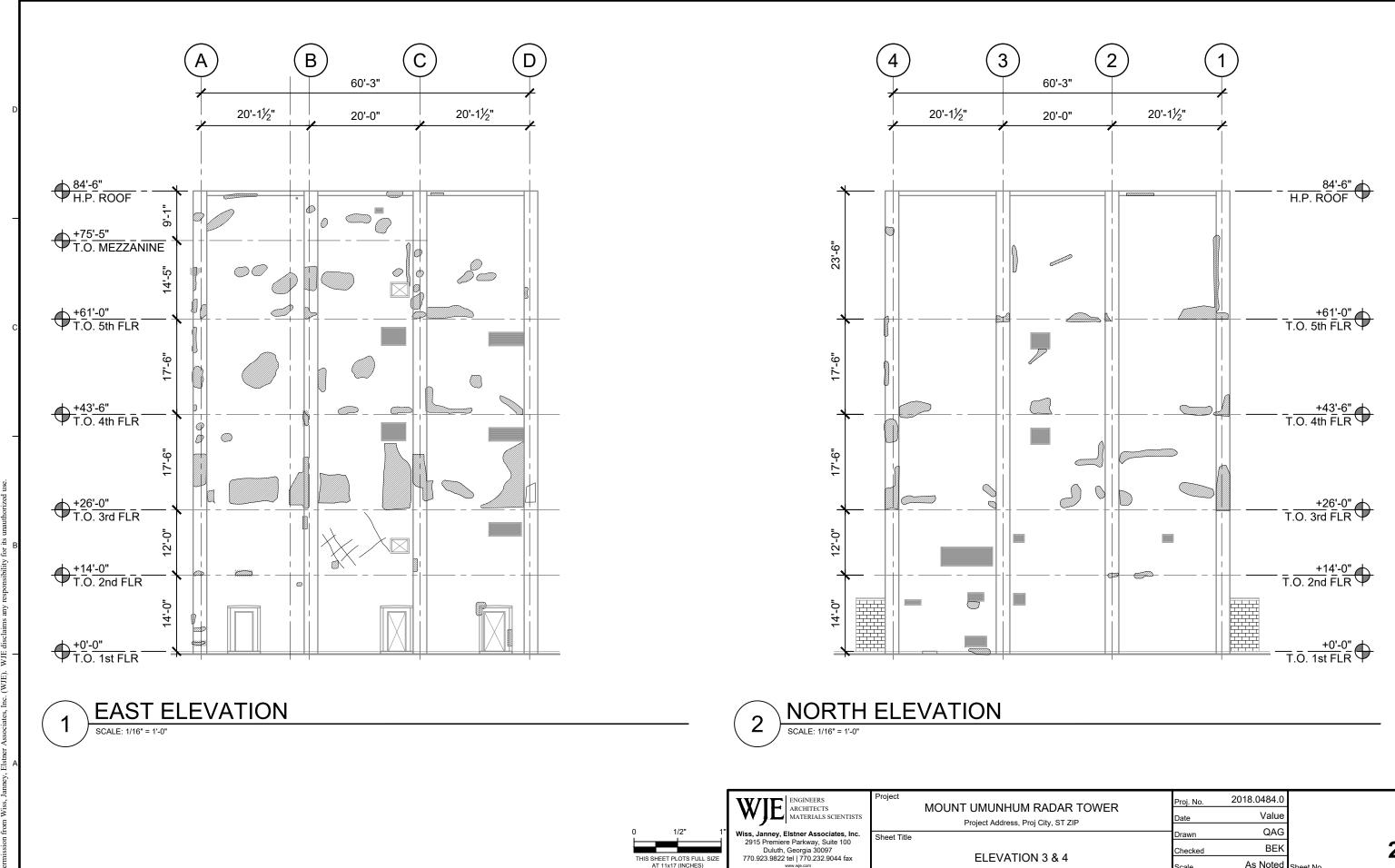
### APPENDIX C: RESULTS OF EXTERIOR WALLS SURVEY



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#### **ATTACHMENT 2 – Recent History**

#### Interim Repair – January 2015

On January 28, 2015, the Board authorized short-term interim structural repairs to the radar tower prior to making a final decision on the future disposition of the structure (R-15-09). The Interim Repair Project included: repairs to the first floor caused by the Loma Prieta earthquake; sealing all first floor openings to prevent the public from entering the structure; and "collapse prevention" repairs required by the County of Santa Clara (County) to allow for public access to the exterior perimeter of the structure as part of the summit grand opening. The Interim Repair Project received final signoff and approval from the County in July 2016. With these Interim Repairs completed, the District moved forward with the summit improvements and opened Mount Umunhum to the public in September 2017. The repairs were designed to be short-term and last at least five years.

#### Retain and Seal – June 2016

On May 10, 2016, the County Board of Supervisors listed the radar tower on the County Heritage Resource Inventory. Given this action and implications on the future disposition of the radar tower, the Board approved the Retain and Seal option for the structure on June 8, 2016 (R-16-75). The Retain and Seal approach effectively calls for the sealing and stabilization of the structure, with no visitor access to the interior.

#### Conservation Easement – December 2017

On December 13, 2017, the Board authorized the Quitclaim of a Cultural Conservation Easement (conservation easement) to protect and preserve the Mount Umunhum Summit in perpetuity (R-17-131). The conservation easement permits the following as it pertains to the radar tower and its future repairs:

"To repair and maintain the Tower, consistent with the District's approved Retain and Seal option and to any degree required by the County of Santa Clara, based on its general health and safety regulatory authority, or based on the structure's status on the County's Heritage Resource Inventory, or as needed in the District's discretion for nature resource management, health and safety purposes, provided in all cases that the Tower is not expanded in footprint or height or use."

#### Exterior Remediation Project – February 2018

Following winter storm events in 2017, flakes of paint from the exterior of the radar tower were observed within the immediate vicinity surrounding the radar tower. To ensure public health and safety, staff immediately closed the area around the radar tower to public access. Simultaneously, the District hired Hazard Management Services, Inc., (HMS) in November 2017 to collect and test samples from the exterior of the radar tower. The collected samples contained low levels of lead, and one sample contained trace amounts of asbestos. Soil samples collected adjacent to the radar tower, however, did not contain hazard levels for heavy metal

contamination, including lead.

On February 14, 2018, the Board authorized funding for the Mount Umunhum Radar Tower Exterior Remediation Project, and approved a contract with PARC Environmental to complete

the removal of all remaining paints and coatings from the exterior four walls of the structure (R-18-18). The Project was completed in May 2018, with two hazardous materials testing firms confirming that all paints had been completely removed from the exterior four faces of the structure.

#### Exterior Assessment Project – July 2018

Following the removal of all exterior paints and coatings, small pieces of aggregate (as large as 1.5 inches in width) were observed around the perimeter of the radar tower. Due to this, the area around the radar tower was fenced off from public access and has been designated a hard-hat required area. Additionally, the closure of the East Summit, Summit Stairs, and Mt. Umunhum Trail from the Summits Stairs to the trailhead shelter remained in effect until a covered pedestrian walkway was installed along the southern face of the radar tower in June 2019. The covered walkway provides protection from falling debris and allows public access to the East Summit from the Summer Shelter.

On July 31, 2018, the District entered into a contract with ZFA Structural Engineers (ZFA) to perform a limited assessment of the radar tower to evaluate the integrity of the exterior concrete walls given the observations of aggregate pieces at the base of the tower. The assessment aimed to determine if the newly exposed concrete walls posed a threat of imminent collapse. ZFA conducted visual observations and hammer tests on the walls to identify concrete and rebar deterioration. The strength and chemical composition of the concrete was determined by extracting and testing core samples. The test results demonstrated the walls to be structurally sound, however, the final report recommended surface repairs of the walls to provide safe public access around the exterior of the radar tower. Findings from ZFA's structural assessment were limited to the exterior walls, and did not provide information on other locations of damage or water intrusion that could lead to continued deterioration of the exterior exposed concrete. In order to provide a long-term repair recommendation to address the deteriorating exterior walls, it was determined that a comprehensive assessment of the entire structure would be required.

#### Comprehensive Repair Project Criteria Approval – September 2018

On September 12, 2018, the Board approved the Repair Project goals, criteria, and scope of work (refer below to the section on Project Goals for details) (R-18-105). The Board-approved scope of work for the Project called for a comprehensive assessment of the radar tower and the development of recommended short and long-term repairs that achieve Board direction.

#### 2019-20 Repair Project – January 2019

On January 23, 2019, the Board authorized the General Manager to enter into a contract with WJE to complete a structural, safety, and hazardous materials assessment of the radar tower, and develop conceptual recommendations for short-term and long-term repair options. The 2019 assessment was designed to build upon the findings and recommendations from the prior exterior assessment completed by ZFA. The WJE project team was selected through a Request for Proposals (RFP) process. The team has prior experience with similar concrete military buildings of historic interest, and expertise in the field of structural engineering and concrete structure evaluation, making WJE a well-qualified and experienced firm to complete the assessment and provide recommended repair options.

#### **BOD Bundled Repair Option Selection – November 2019**

On November 20, 2019, the Board selected the Long-Term Repairs option that included cost effective repair options to stabilize and extend the life span of the radar tower, and directed the General Manager to return to the Board with a contract recommendation to develop construction documents for the selected repair option.

#### BOD Roof Repair Options Peer-Review – January 2020

On January 6, 2020, the District entered into an on-call contract with Mesiti-Miller Engineering, Inc., (MME) to provide structural engineering services. The first task issued to MME under the on-call contract was to perform a peer-review of the roof repairs proposed by WJE in the BOD, and determine if a spray-on foam roof would be a suitable long-term repair option for the structure. The MME peer-review concludes that the installation of a new roofing system as described in the BOD would provide a stable long-term product and be more economical in the long run compared to a spray-on foam roof. Before a spray-on foam roof could be applied, substantial repairs of the existing failing concrete roof would be required in order to provide a suitable surface that would be able to accept a foam roof material to ensure waterproofing. With these repairs to establish a sound surface for application (and also address water drainage), the total cost associated with a foam roof would surpass the cost for the recommended new roofing system.